

Chambers Road Apartments Traffic Impact Study

City of Commerce City



Date: June 11, 2024

Submitted To:

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1.0 EXECUTIVE SUMMARY

This traffic impact study has been prepared by the Fox Tuttle Transportation Group for the Chambers Road Apartments project. The project proposes to develop multifamily residential use on a ±12.7-acre site in the City of Commerce City. The project parcel is located along the west side of Chambers Road approximately 1,000' south of E. 104th Avenue. The purpose of this study is to assist the City in identifying potential traffic impacts within the study area with buildout of this project in the short-term (Year 2027) and long-term, 20-year (Year 2044) planning horizons.

The project is proposing to construct 283 apartment units on the site. Access is proposed via an extension of E. 102nd Avenue west from Chambers Road, with two full-movement access points into the project site from E. 102nd Avenue: 1) located approximately 370' west of Chambers Road and, 2) aligning with Altura Street approximately 1,200' west of Chambers Road.

The Chambers Road Apartments project is anticipated to generate 1,907 daily, 113 AM peak hour, and 144 PM peak hour trips at buildout. These volumes will be distributed along Chambers Road north (to/from E. 104th Avenue) and south (to/from E. 96th Avenue) and along E. 102nd Avenue, which will ultimately connect to Sable Blvd. with the development of Anderson Ranch to the west.

Based on the projected rate of background (non-project) within the study area, it is anticipated that the following background (non-project related) improvements will need to be in place by the Year 2044 scenario to service existing volumes and future area growth at acceptable Levels of Service per City criteria:

Long-Term (Year 2044) Background (non-project related) Improvement Recommendations:

- If future regional growth projections are realized, the intersection of E. 104th Avenue & Chambers Road will need to be constructed to its buildout lane geometry, with three (3) through lanes in each direction eastbound and westbound on E. 104th Avenue and exclusive right-turn lanes added on the eastbound, northbound and southbound approaches.
- If future regional growth projections are realized, the intersection of E. 96th Avenue & Chambers Road will need to be constructed to its buildout lane geometry, with two (2) through lanes in each direction eastbound and westbound on E. 96th Avenue.
- Roadway improvements identified by the adjacent Anderson Ranch project were assumed to be implemented in the background for this study. Those improvements include:

- Install traffic signal control at E. 104th Avenue and Sable Boulevard. Extend northbound left turn storage at the intersection to 125 feet.
- Add eastbound, northbound, and southbound left turn auxiliary lanes and southbound right turn auxiliary lane at E. 100th Avenue and Chambers Road. Install traffic signal control at the intersection.
- Extend eastbound left turn storage at E. 96th Avenue & Chambers Road to 425 feet. Extend southbound right turn storage at E. 96th Avenue & Chambers Road to 175 feet.

The project-added traffic can be accommodated on the existing and proposed roadway and intersection network with the following recommended improvements in place:

Project-Related Improvement Recommendations (Responsibility of the Developer):

- Extend E. 102nd Avenue from Chambers to the west along the project frontage. This roadway should be built consistent with the City's "Minor/Residential Collector" standards per the C3 Vision Transportation Plan Roadway Classification Plan.
- The intersection of E. 102nd Avenue should include a northbound left-turn deceleration lane (175' lane + 180' taper) and a southbound right-turn deceleration lane (135' + 180'), consistent with the City of Commerce City Engineering Construction Standards and Specifications. The eastbound approach should be striped with a 50' left turn lane.
- Construct the east access to the site along E. 102nd Avenue with side-street stop-sign control on the southbound approach. No auxiliary turn lanes are required or recommended at this access given the Minor Collector roadway classification, consistent with City standards.
- Construct the west access to the site along E. 102nd Avenue to align with Altura Street, with side-street stop-sign control on the northbound and southbound approaches. No auxiliary turn lanes are required or recommended at this access given the Minor Collector roadway classification, consistent with City standards.

CHAMBERS ROAD APARTMENTS TRAFFIC IMPACT STUDY

2.0 INTRODUCTION

This traffic impact study has been prepared by the Fox Tuttle Transportation Group for the Chambers Road Apartments project in the City of Commerce City. The project proposes to construct 283 apartment units. The project parcel is located in the northwest quadrant of Chambers Road and E. 102nd Avenue alignment, approximately 1,000 feet south of E. 104th Avenue.

The purpose of this study is to assist in identifying potential traffic impacts within the study area with buildout of this project in the short-term (Year 2027) and long-term (Year 2044) scenarios. The traffic study addresses peak hour intersection conditions in the study area without and with the project added traffic. The information contained in this study is anticipated to be used by the City of Commerce City in identifying any intersection or roadway deficiencies and potential improvements for both the near term and long-term future scenarios.

3.0 PROJECT DESCRIPTION

The project is proposing to develop 283 apartment units on a ±12.7-acre site which currently has a single residential use. The project proposes to extend E. 102nd Avenue along the project's southern frontage. Access to the site is proposed, as follows:

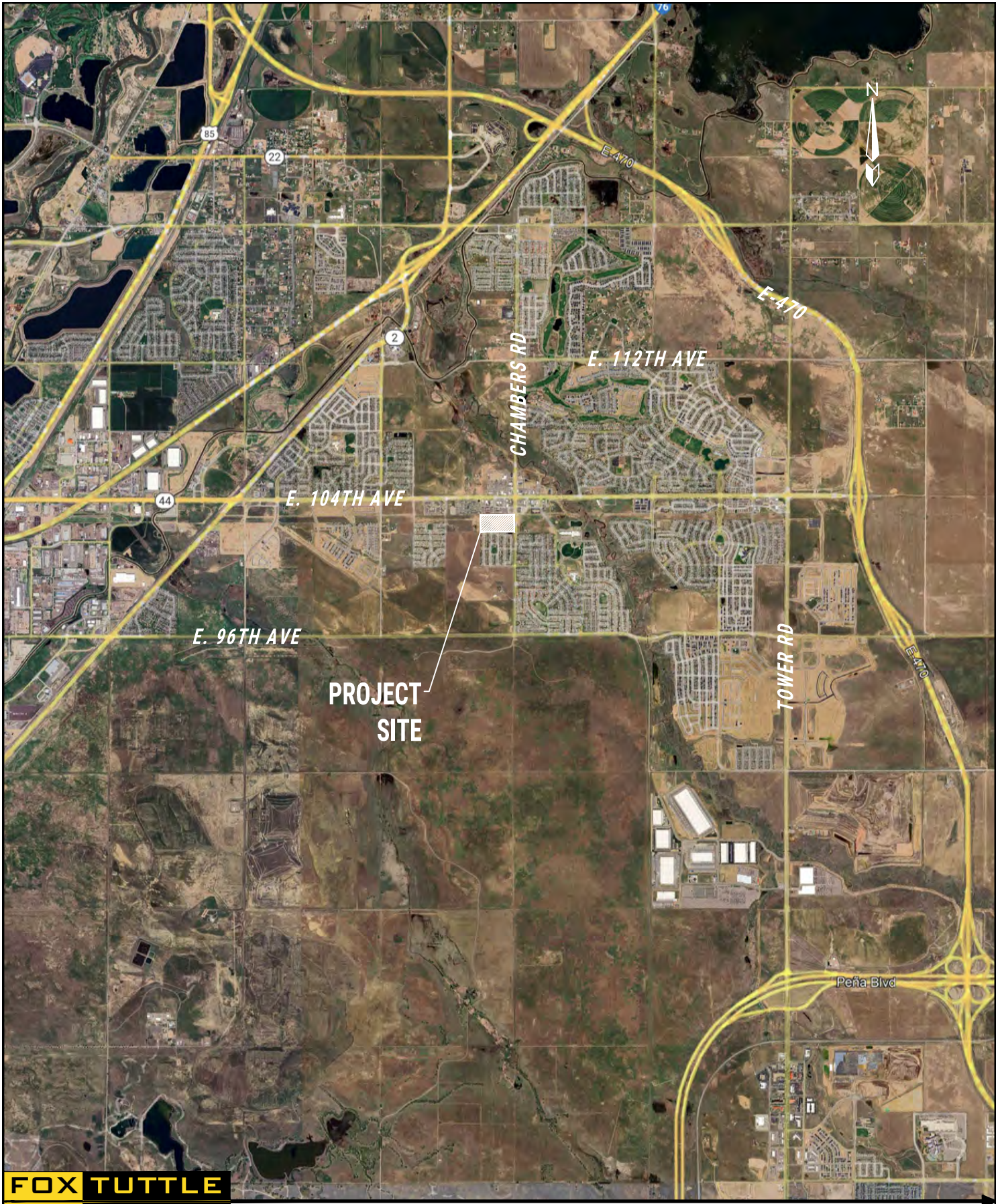
- Full-movement access along E. 102nd Avenue approximately 370' west of Chambers Road
- Full-movement access aligning with Altura Street approximately 1,200' west of Chambers Road

Development of the project includes construction of the E. 102nd Avenue & Chambers Road intersection as a full-movement, side-street stop-sign controlled intersection, which will provide access to the project via Chambers Road. Additional access to the site to the west will be provided via the ultimate connection of E. 102nd Avenue with development of the adjacent Anderson Ranch project.

The project site has been identified as a "Residential High" future land use per the C3 Vision Plan (2010). The proposed use is consistent with this zoning designation. Adjacent land uses include existing single-family residential use to the south in the Harvest Meadows

neighborhood, future attached and detached single-family residential uses to the south and west in the Anderson Ranch neighborhood, and multifamily apartments and mixed-use commercial to the north of the site.

A vicinity map is shown on **Figure 1**. A concept site plan depicting the proposed access points is shown on **Figure 2**.



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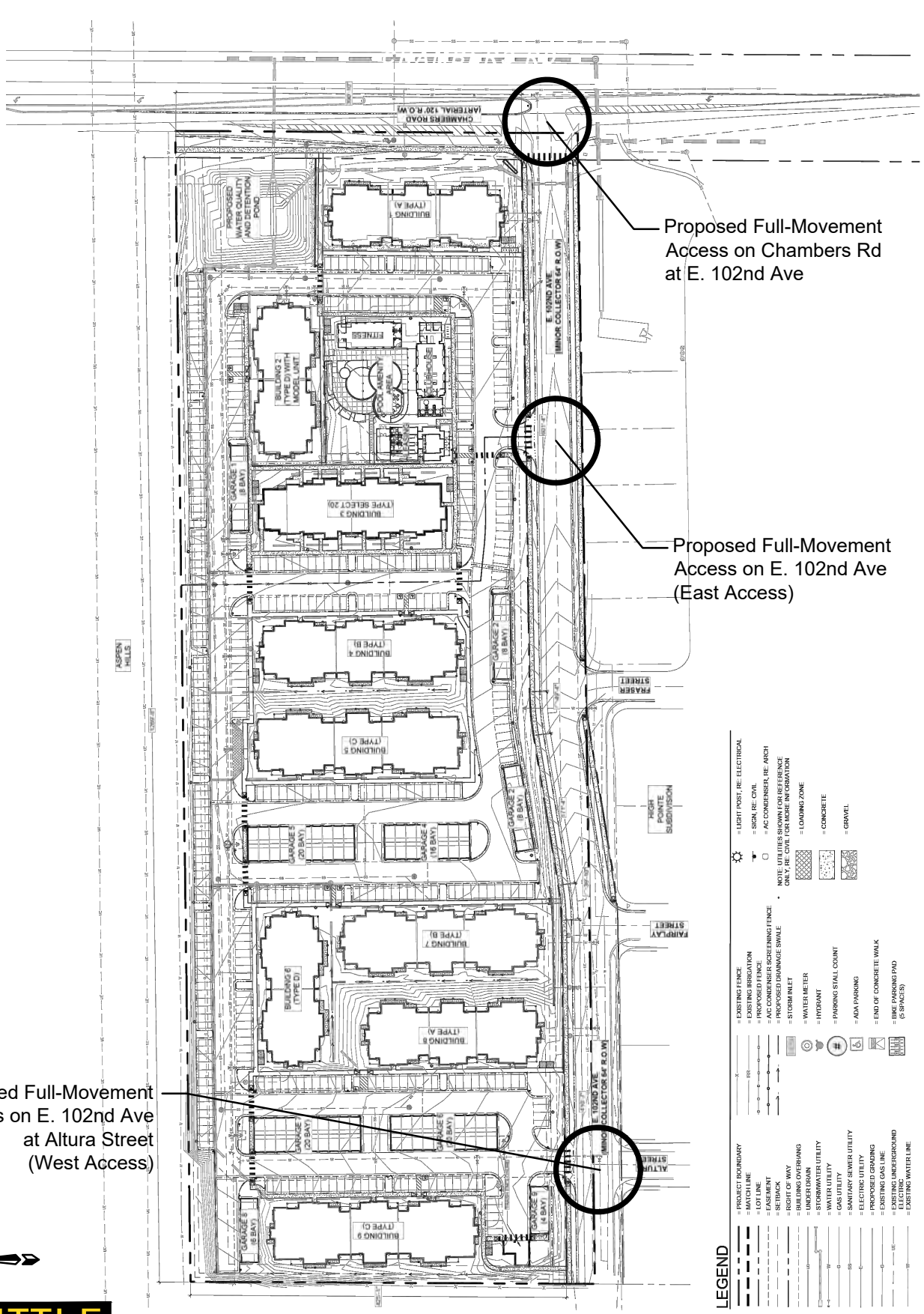
TITRES AT COMMERCE CITY CHAMBERS ROAD DEVELOPMENT PLAN

A PART OF THE SOUTH HALF (S1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION EIGHTEEN (18), TOWNSHIP TWO (2) SOUTH, RANGE SIXTY-SIX (66) WEST OF THE 6TH P.M. CASE # (D24-0006), PARCEL #0172300000150

Proposed Full-Movement Access on E. 102nd Ave at Altura Street (West Access)

Proposed Full-Movement Access on Chambers Rd at E. 102nd Ave

Proposed Full-Movement Access on E. 102nd Ave (East Access)



LEGEND

- PROJECT BOUNDARY
- MATCH LINE
- EXISTING
- EXISTING
- SE-TACK
- RIGHT OF WAY
- BUILDING OVERLAP
- UNDER DRAIN
- WATER UTILITY
- GAS UTILITY
- SANITARY SEWER UTILITY
- ELECTRICAL UTILITY
- PROPOSED GRADING
- EXISTING GAS LINE
- EXISTING UNDERGROUND (S SPACES)
- EXISTING WATER LINE
- EXISTING FENCE
- EXISTING IRRIGATION
- PROPOSED SCREENING FENCE
- PROPOSED DRAINAGE SWALE
- STORM INLET
- WATER METER
- HYDRANT
- ADA PARKING
- PARKING STALL COUNT
- END OF CONCRETE WALK
- BIKE PARKING PAD (S SPACES)
- LIGHT POST, RE-ELECTRICAL
- SIGN, RE-CIVIL
- AC CONDENSER, RE-ARCH
- NOTE: UTILITIES SHOWN FOR REFERENCE ONLY, RE-CIVIL FOR MORE INFORMATION
- LOADING ZONE
- CONCRETE
- GRAVEL

4.0 EXISTING AND FUTURE BACKGROUND TRAFFIC CONDITIONS

4.1 Study Area and Circulation Network

The study area boundaries took into consideration the amount of traffic to be generated by the project and potential impact to the existing and proposed roadway network.

The existing study area street network consists of arterial, collector and local streets. The primary public roadways that will provide access to the project site are discussed in the following text. Roadway classifications discussed are consistent with the City of Commerce City C3 Vision Transportation Plan¹. The existing study area roadway network is illustrated on **Figure 1**.

Chambers Road is a two-lane “Multimodal Arterial” providing north-south access through the study area, extending from E. 96th Avenue to the south to E. 120th Avenue to the north. Chambers Road has a posted speed of 40 miles per hour (mph) adjacent to the project site. There exists a 12’ concrete multi-use path along the east side of Chambers Road between E. 96th Avenue and E. 104th Avenue and a 6’ detached sidewalk on the west side of Chambers Road extending from roughly 200’ north of the site to E. 104th Avenue. Chambers Road widens to a four-lane section with auxiliary lanes north of the site within commercial areas adjacent to E. 104th Avenue. The current limits of the four-lane section were assumed to remain the same for future conditions in this study.

E. 104th Avenue is a four-lane, “Principal Arterial” roadway that provides east-west access north of the project site, extending from E. 470 on the east to US 36 and Westminster to the west. E. 104th Avenue has a posted speed of 45 mph within the study area. The Chambers Road & E. 104th Avenue intersection is controlled with a traffic signal. The C3 Vision Transportation Plan identifies E. 104th Avenue to be built to six (6) lanes at buildout, with three (3) through lanes in each direction, in addition to auxiliary turn lanes. Transit service exists on E. 104th Avenue at Chambers Road via the RTD 104L route, which provides access between Wagon Road Park-n-Ride and Denver Airport Station, with connectivity to other regional routes.

E. 96th Avenue is a two-lane, “Minor Arterial” roadway that provides east-west access south of the project site, extending from E. 470 on the east to I-76 to the west. E. 96th Avenue has a posted speed of 30 mph within the study area. The Chambers Road & E. 96th Avenue intersection is controlled with a traffic signal. The C3 Vision Transportation Plan identifies E. 96th Avenue to be built to four (4) lanes at buildout, with two (2) through lanes in each direction, in addition to auxiliary turn lanes.

¹ C3 Vision Transportation Plan. City of Commerce City. July 2010.

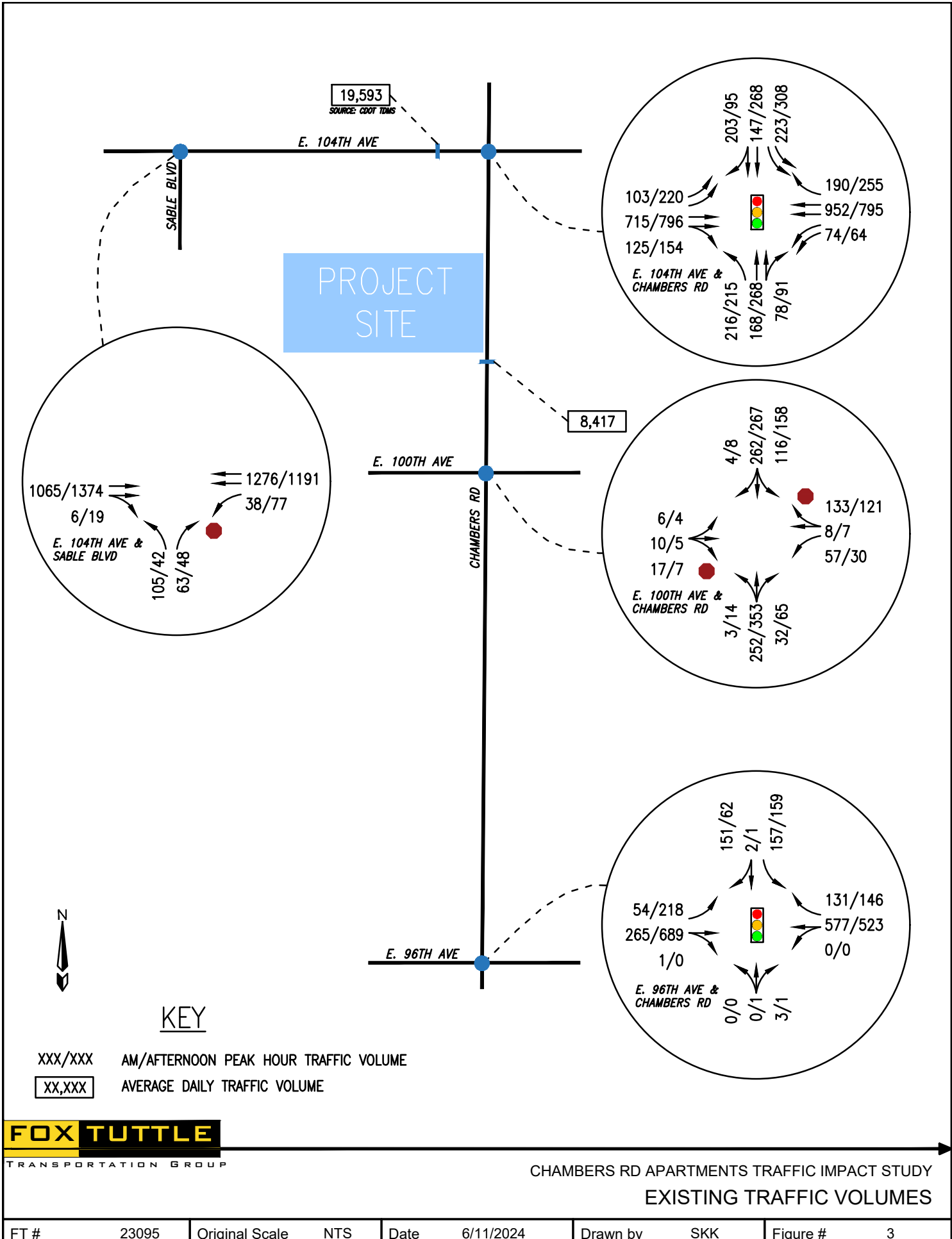
E. 102nd Avenue is a two-lane “Minor/Residential Collector” roadway that will provide east-west access to existing and proposed residential land uses in the immediate project site area. Only a short segment of E. 102nd Avenue exists today, on the north border of the existing Harvest Meadows neighborhood to the south of the project site. With development of the adjacent Anderson Ranch project, E. 102nd Avenue will be extended/improved to Sable Blvd to the west. The Chambers Road Apartments project proposes to construct the section of E. 102nd Avenue, extending from Chambers Road to the west along the project’s southern frontage. This roadway will be a two-lane street consistent with the C3 Vision Transportation Plan.

E. 100th Avenue is a two-lane “Minor/Residential Collector” roadway that provides east-west access in the project area. The adjacent Anderson Ranch project will complete the missing south half of E. 100th Avenue west of Chambers Road and extend E. 100th Avenue to Sable Boulevard. Anderson Ranch also plans to install traffic signal control

Sable Boulevard is a two-lane “Minor/Residential Collector” roadway that provides north-south access to developments south of 104th Avenue. The adjacent Anderson Ranch project will extend Sable Boulevard from its current limits south to 98th Avenue (also to be built by Anderson Ranch). Anderson Ranch also plans to install traffic signal control at E. 104th Avenue & Sable Boulevard and extend the northbound auxiliary lane storage.

4.2 Existing Traffic Volumes

Weekday AM and PM peak hour traffic volumes and daily traffic volumes were collected at study area intersections in November 2023. The existing traffic volumes are illustrated on **Figure 3**, along with the existing intersection geometry and traffic controls. Count data sheets are provided in the Appendix.



4.3 Annual Growth Factor and Future Volume Methodology

Potential for background (non-project) traffic growth in the study area was considered based on a review of Denver Regional Council of Governments (DRCOG) regional travel model projections and Colorado Department of Transportation (CDOT) growth factors. Additional trip projections from the adjacent Anderson Ranch Traffic Impact Study (Kimley-Horn, January 2023) were also incorporated. Using the DRCOG 2050 model projections vs. 2020 base-year data, it was determined that an average 2.1% annual background growth would effectively represent traffic volume growth along Chambers Road and at study area intersections over the next 20 years, with the site-specific projections of the Anderson Ranch buildout added on top of this ambient growth.

Using this methodology, the Year 2027 (short-term planning horizon) and Year 2044 background (without project) traffic volumes are provided on **Figure 4** and **Figure 5**, respectively.

4.4 Existing, Year 2027 Background, and Year 2044 Background Capacity Analysis

Existing, Year 2027 and Year 2044 traffic scenarios *without* development of the project were analyzed to provide a baseline for future year comparison.

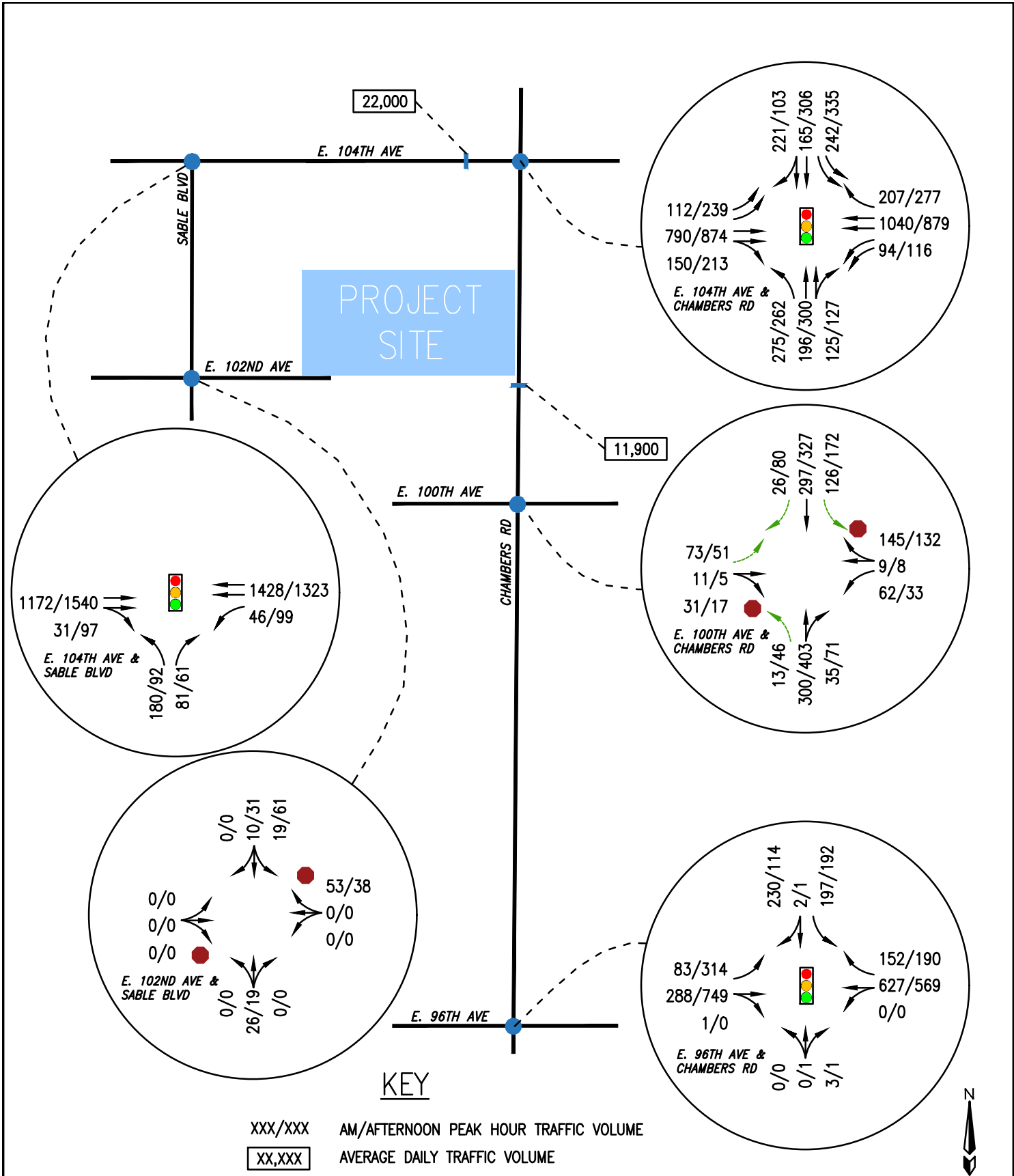
In determining the operational characteristics of an intersection, “Levels of Service” (LOS) A through F are applied, with LOS A indicating very good operations and LOS F indicating congested operations. The intersection LOS is represented as a delay in seconds per vehicle for the intersection as a whole and for each turning movement. A more detailed discussion of LOS methodology is contained in the Appendix for reference. Criteria contained in the Highway Capacity Manual (HCM), using Synchro software, was applied for these analyses in order to determine LOS and 95th-percentile queues during peak hour periods. Traffic signal phasing and timing operations were obtained from City staff and incorporated into the modeling.

The results of the LOS and queue calculations for the baseline traffic scenarios (without project) are summarized on **Table 1**. The intersection LOS and queue worksheets are attached in the Appendix. The data in the tables shows that all study intersections are operating acceptably and will continue to operate acceptably (overall LOS D or better) in the AM and PM peak hours for all existing, short-term and long-term background scenarios.

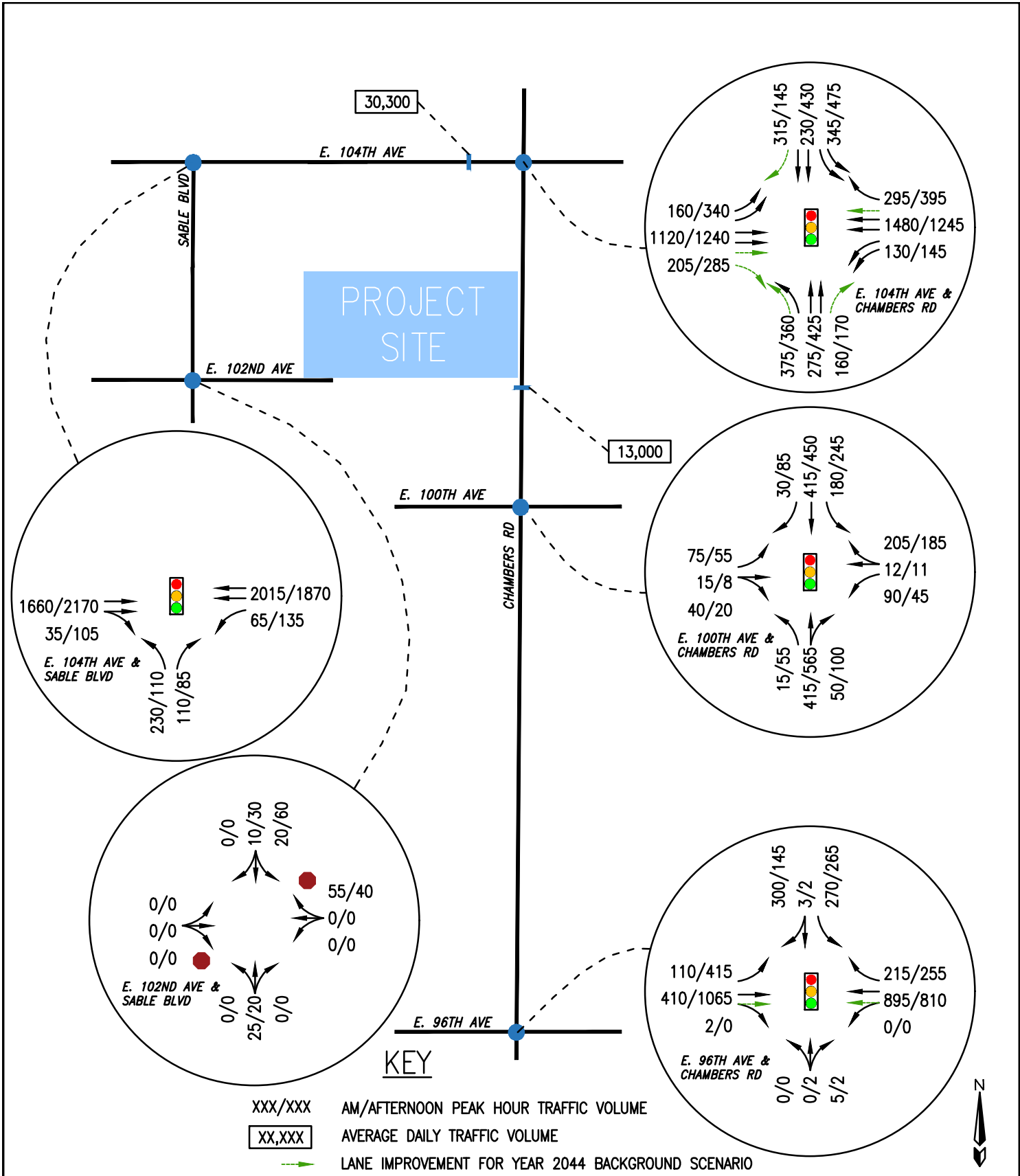
Some individual left-turn movements are shown to operate at LOS E in existing and future scenarios. This level of delay is typical of protected-only left-turn operation with a long (120-second) cycle length, and is in place to maximize safety.

The following background (non-project related) long-term future roadway improvements were identified as necessary to maintain LOS D or better overall for the Year 2044 scenario:

- E. 104th Avenue & Chambers Blvd. will need to be widened to its buildout lane geometry, with three (3) through lanes in each direction eastbound and westbound on E. 104th Avenue and exclusive right-turn lanes added on the eastbound, northbound and southbound approaches; northbound left-turn queues are shown to exceed storage lengths in the Year 2044 background condition, though there is sufficient capacity in the northbound approach to accommodate these queues without impacts to adjacent accesses/intersections.
- E. 96th Avenue & Chambers Blvd. will need to be widened to its buildout lane geometry, with two (2) through lanes in each direction eastbound and westbound on E. 96th Avenue; in addition, the queue analysis shows the eastbound left-turn and southbound left-turn storage lengths should be extended as the intersection is improved to accommodate existing and future queues.



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Table 1 – Existing and Future Background Intersection LOS and Queue Summary

Int.	Lane	Storage	Existing						Year 2027 Background						Year 2044 Background					
			AM Peak			PM Peak			AM Peak			PM Peak			AM Peak			PM Peak		
			Delay (s)	LOS	Q ⁽¹⁾	Delay (s)	LOS	Q ⁽¹⁾	Delay (s)	LOS	Q ⁽¹⁾	Delay (s)	LOS	Q ⁽¹⁾	Delay (s)	LOS	Q ⁽¹⁾	Delay (s)	LOS	Q ⁽¹⁾
Chambers Rd & 104th Ave	Overall	-	38	D	-	45	D	-	46	D	-	41	D	-	35	C	-	38	D	-
	EB LT	275'	60	E	72'	65	E	151'	60	E	85'	62	E	155'	58	E	80'	63	E	221'
	EB TH+RT	-	27	C	351'	46	D	726'	40	D	400'	33	C	430'	20	B	256'	25	C	283'
	EB RT	-													18	B	33'	24	C	48'
	WB LT	380'	61	E	55'	54	D	78'	61	E	66'	60	E	77'	61	E	89'	61	E	97'
	WB TH	-	29	C	422'	37	D	537'	48	D	613'	33	C	493'	24	C	449'	32	C	424'
	WB RT	350'	0	A	34'	0	A	27'	0	A	43'	0	A	27'	0	A	55'	0	A	76'
	NB LT	290'	71	E	268'	47	D	289'	69	E	252'	46	D	263'	40	D	170'	37	D	135'
	NB TH+RT	-	43	D	125'	48	D	217'	37	D	129'	48	D	207'	48	D	157'	53	D	226'
	NB RT	-													59	E	61'	52	D	63'
	SB LT	285'	38	D	107'	36	D	131'	31	C	99'	37	D	127'	40	D	160'	40	D	181'
SB TH+RT	-	61	E	112'	57	E	217'	50	D	108'	54	D	208'	46	D	136'	48	D	225'	
SB RT	-													80	E	271'	31	C	64'	
Chambers Rd & 96th Ave	Overall		20	C	-	28	C	-	23	C	-	23	C	-	23	C	-	29	C	-
	EB LT	70'	29	C	64'	29	C	207'	32	C	68'	29	C	194'	31	C	101'	64	E	438'
	EB TH+RT	-	7	A	124'	21	C	536'	7	A	135'	10	A	463'	9	A	88'	9	A	256'
	WB LT+TH	-	23	C	544'	54	D	634'	28	C	618'	39	D	599'	26	C	392'	33	C	353'
	WB RT	-	12	B	43'	16	B	59'	12	B	43'	15	B	46'	18	B	51'	25	C	57'
	NB LT+TH+RT	-	51	D	0'	60	E	6'	53	D	0'	59	E	6'	48	D	0'	53	D	8'
	SB LT	85'	27	C	141'	29	C	160'	28	C	154'	29	C	153'	24	C	242'	39	D	259'
	SB TH+RT	-	28	C	39'	24	C	33'	30	C	40'	24	C	32'	34	C	64'	28	C	48'
Chambers Rd & 100th Ave	Overall		5	A	-	5	A	-	9	A	-	10	B	-	19	B	-	12	B	-
	EB LT	150'							62	F	73'	>120	F	90'	56	E	162'	20	B	33'
	EB LT+TH+RT	-	17	C	8'	24	C	8'	14	B	8'	17	C	5'	38	D	46'	14	B	15'
	WB LT	100'	25	D	25'	34	D	20'	36	E	40'	62	F	38'	42	D	118'	15	B	27'
	WB TH+RT	-	12	B	23'	14	B	25'	13	B	28'	16	C	35'	45	D	77'	18	B	50'
	NB LT	235'												10	A	10'	8	A	22'	
	NB LT+TH+RT	-	8	A	0'	8	A	0'	8	A	0'	8	A	3'	8	A	190'	9	A	307'
	SB LT	235'												15	B	201'	26	C	178'	
SB LT+TH+RT	-	8	A	8'	9	A	13'	8	A	10'	9	A	15'	8	A	394'	7	A	137'	
SB RT	135'												6	A	25'	4	A	15'		
Sable Blvd & 104th Ave	Overall		29	C	-	11	B	-	11	B	-	8	A	-	13	B	-	11	B	-
	EB TH+RT	-	0	A	0'	0	A	0'	7	A	304'	6	A	394'	8	A	303'	7	A	369'
	WB LT	300'	12	B	5'	15	B	18'	10	B	9'	18	B	58'	19	B	37'	110	F	282'
	WB TH	-	0	A	0'	0	A	0'	7	A	126'	4	A	78'	9	A	219'	4	A	557'
	NB LT	125'	>120	F	285'	>120	F	138'	59	E	214'	62	E	128'	62	E	263'	62	E	147'
NB RT	-	14	B	13'	17	B	13'	49	D	53'	58	E	69'	47	D	116'	59	E	112'	
Sable Blvd & 102nd Ave	Overall								6	A	-	5	A	-	6	A	-	5	A	-
	EB LT+TH+RT	-							0	A	0'	0	A	0'	0	A	0'	0	A	0'
	WB LT+TH+RT	-							9	A	5'	9	A	3'	9	A	5'	9	A	3'
	NB LT+TH+RT	-							0	A	0'	0	A	0'	0	A	0'	0	A	0'
SB LT+TH+RT	-							7	A	0'	7	A	3'	7	A	0'	7	A	3'	

⁽¹⁾ 95th-percentile queue length as calculated by Synchro/HCM

Note: Delay represented in average seconds per vehicle.

5.0 PROPOSED DEVELOPMENT TRAFFIC

5.1 Trip Generation

In order to estimate the anticipated volume of trips generated by the site at buildout, trip rates contained in the Institute of Transportation Engineers (ITE) Trip Generation manual² for the ITE #220 “Multifamily Housing (Low-Rise)” land use category were applied, consistent with the requirements of Section 5.02.1.D. of the City of Commerce City Engineering Construction Standards and Specifications.

The resulting existing trip estimates using ITE rates are provided on **Table 2**, below.

Table 2 – Trip Generation Estimates

Land Use	Size	Unit	Average Daily Trips				AM Peak Hour Trips				PM Peak Hour Trips			
			Rate	Total	In	Out	Rate	Total	In	Out	Rate	Total	In	Out
ITE #220 - Multifamily Housing (Low-Rise)	283	Dwelling Units	6.74	1907	954	953	0.40	113	27	86	0.51	144	91	53
Total Added Trips				1907	954	953		113	27	86		144	91	53

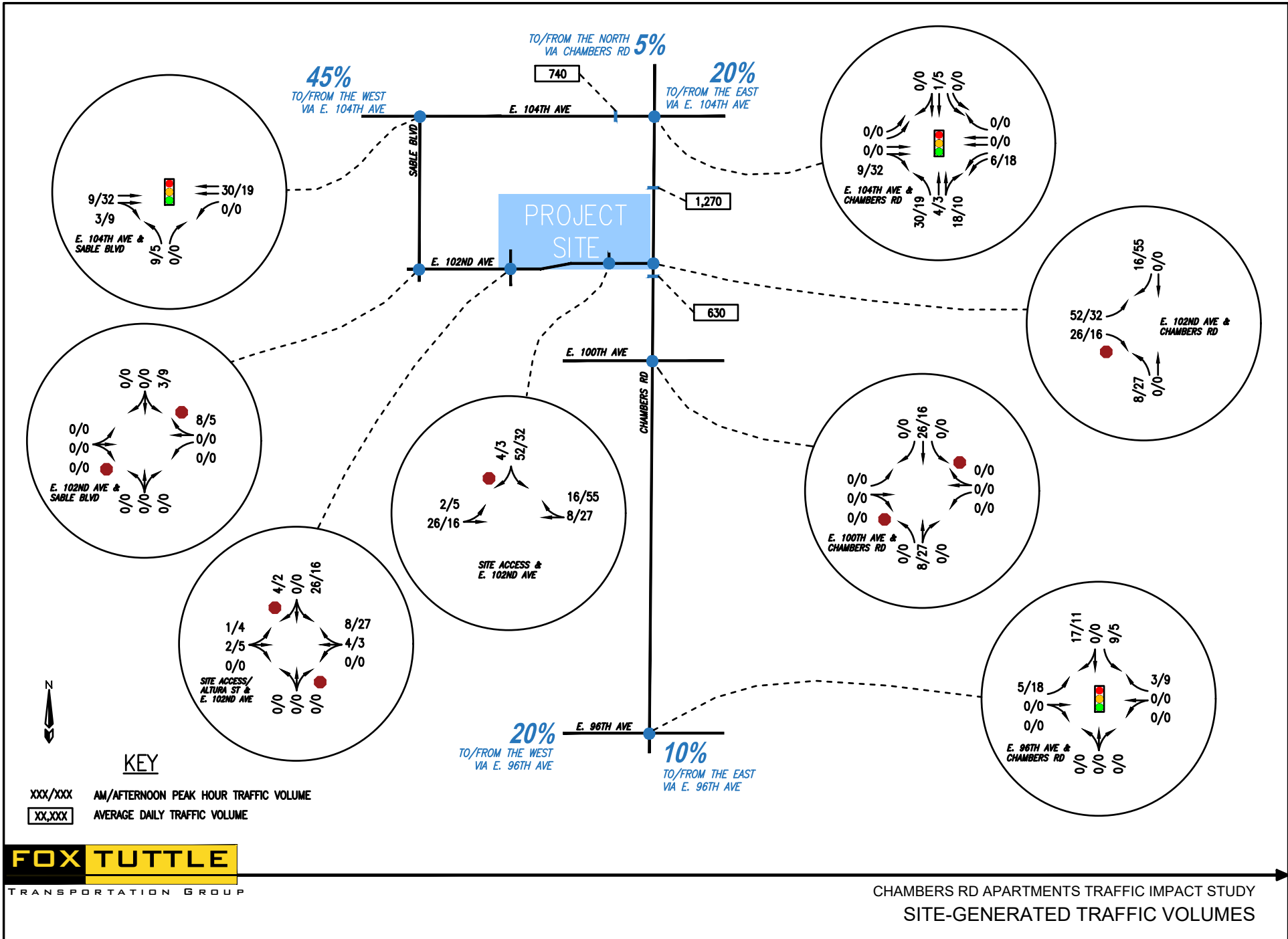
Source: ITE Trip Generation 11th Edition. 2021.

As show above, using ITE rates the project site is anticipated to generate approximately 1,907 daily, 113 AM peak hour and 144 PM peak hour trips at full buildout and occupancy.

5.2 Trip Distribution and Assignment

The site-generated traffic volumes presented in **Table 2** were distributed onto the adjacent street network based on existing traffic data, existing and anticipated future traffic patterns in and around the area, and consideration of local and regional origins and destinations for the proposed residential use. The assumed distribution percentages are shown on **Figure 6**, along with the resulting site-generated traffic volume estimates with buildout of the project.

² Trip Generation 11th Edition, Institute of Transportation Engineers, 2021.



CHAMBERS RD APARTMENTS TRAFFIC IMPACT STUDY
SITE-GENERATED TRAFFIC VOLUMES

FT Project #	23095	Original Scale	NTS	Date	6/11/2024	Drawn by	SKK	Figure #	6
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6.0 FUTURE TRAFFIC CONDITIONS WITH SITE DEVELOPMENT

This analysis has been conducted in order to determine impacts associated with full development and occupancy of the project in the short-term and long-term scenarios.

With the connectivity of E. 102nd Avenue to Chambers Road, it is anticipated that some existing traffic from the western portion of the Harvest Meadows neighborhood to the south, as well as a portion of future trips from the Anderson Ranch development to the west (on the order of a total of 5-10 trips per peak hour based on the number of residences these routes would serve), may shift to this connection as it will provide more direct access to Chambers Road and the E. 104th Avenue corridor. The future-year projections analyzed for the 2027 and 2044 scenarios with the project development in place assume this shift.

6.1 Intersection Capacity Analysis for Year 2027 w/Project Scenario

The site-generated traffic volumes were added to the Year 2027 background traffic volumes to analyze potential site impacts in the short-term buildout scenario, representing full buildout of the site. The Year 2027 total (with project) traffic volumes are illustrated on **Figure 7**. The level of service criteria discussed in prior sections was applied to the study area intersections to determine impacts with the addition of site-build out traffic volumes in the short-term planning horizon. The results of the LOS and queue calculations for the intersections are summarized for each scenario on **Table 3**. The intersection level of service and queue worksheets are attached in the Appendix.

As shown in **Table 3**, the study area intersections and proposed accesses are projected to operate acceptably (LOS D or better overall) with the addition of site traffic at full project buildout in the short-term.

The E. 102nd Avenue & Chambers Road intersection is anticipated to operate acceptably overall as a side-street stop-controlled intersection, with left-turn deceleration and right-turn deceleration lanes in place, consistent with City of Commerce City Engineering Construction Standards and Specifications requirements (discussed further in Section 6.3).

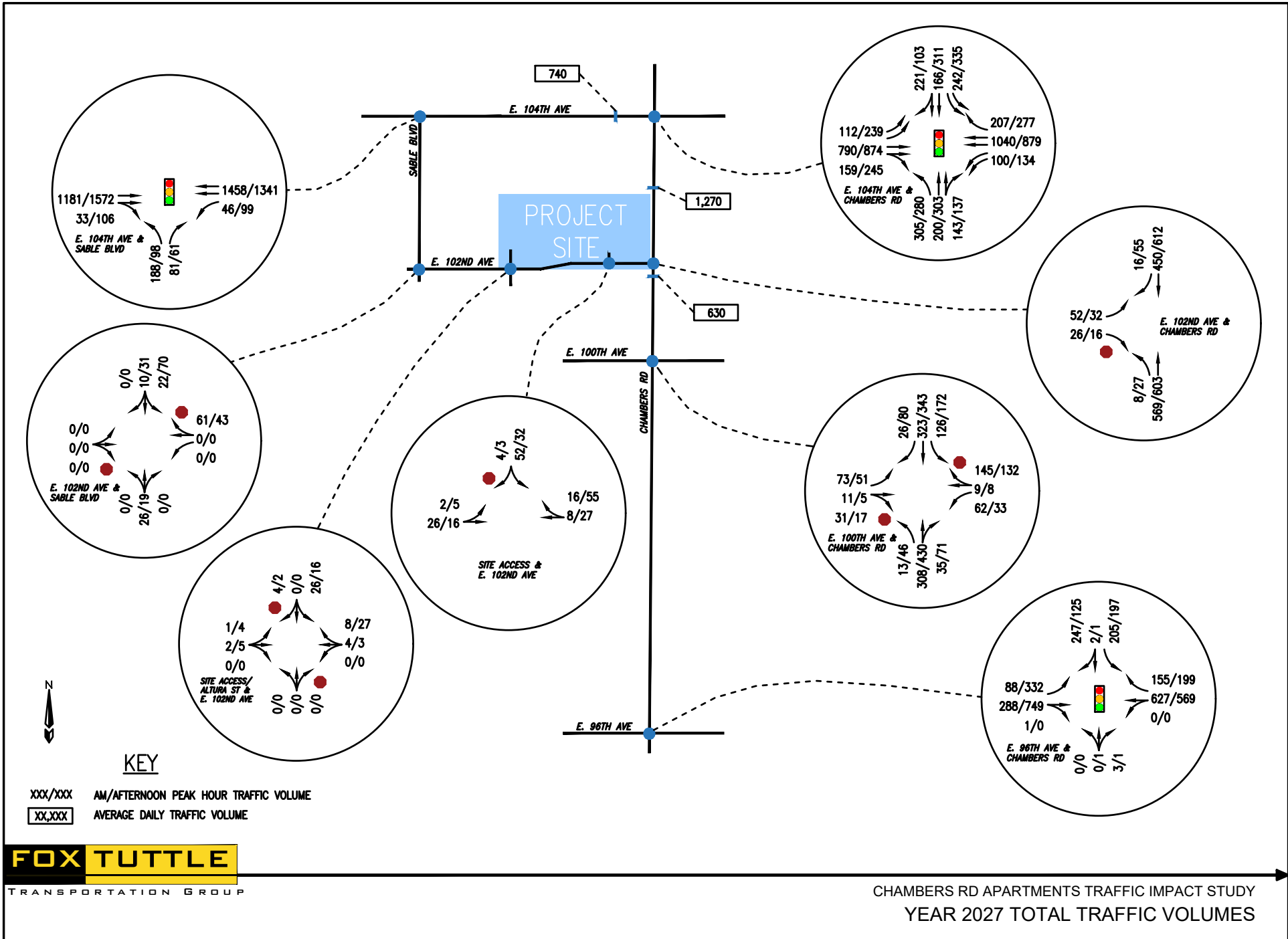
6.2 Intersection Capacity Analysis for Year 2044 w/Project Scenario

The site-generated traffic volumes were added to the Year 2044 background traffic volumes to analyze potential site impacts in the long-term buildout scenario. The Year 2044 total (with project) traffic volumes are illustrated on **Figure 8**. The level of service criteria discussed in prior sections was applied to the study area intersections to determine impacts with the addition of site-build out traffic volumes in the long-term planning horizon. The results of the LOS and

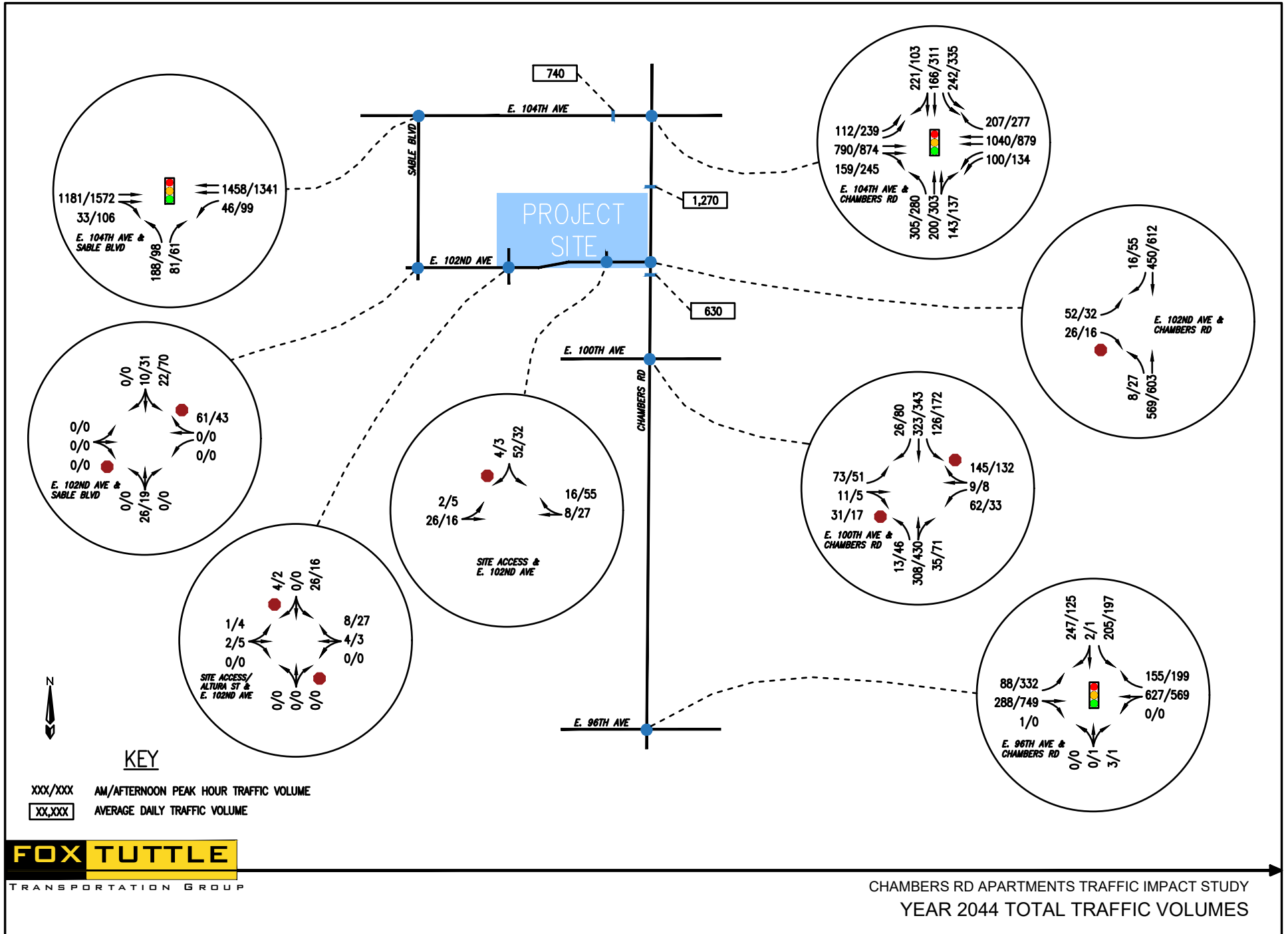
queue calculations for the intersections are summarized for each scenario on **Table 3**. The intersection level of service and queue worksheets are attached in the Appendix. Long-term background improvements, including the widening of E. 104th Avenue to six-lanes and E. 96th Avenue to four-lanes, to meet 20-year background growth projections, were assumed in place for this scenario.

As shown in **Table 3**, the study area intersections and proposed accesses are projected to operate acceptably overall (LOS D or better) with the addition of site traffic with full project buildout in the long-term. The E. 102nd Avenue & Chambers Road intersection is anticipated to continue to operate acceptably overall as a side-street stop-controlled intersection, with left-turn deceleration and right-turn deceleration lanes in place, consistent with City of Commerce City Engineering Construction Standards and Specifications requirements (discussed further in Section 6.3).

Although the intersection is anticipated to operate at acceptable overall LOS, the individual eastbound left-turn movement at E. 102nd Avenue & Chambers Road is anticipated to operate at LOS F in the AM and PM peak hour for 20-year buildout conditions. The projected 95th-percentile queues during these periods is calculated to be 1.9 to 2.5 vehicles, and increase in queue of approximately 1 vehicle per peak hour from background conditions, indicating minimal operational impact with the addition of project traffic. This LOS is common of a side-street stop-controlled approach to an arterial roadway and does not in itself warrant mitigation. However, traffic signal warrants were reviewed to determine if this intersection would approach signal warrant thresholds in the Year 2044 buildout scenario. As discussed in Section 6.4, this intersection is not anticipated to meet signal warrants and can operate effectively with side-street stop control.



FT Project #	23095	Original Scale	NTS	Date	6/11/2024	Drawn by	SKK	Figure #	7
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FT Project #	23095	Original Scale	NTS	Date	6/11/2024	Drawn by	SKK	Figure #	8
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Table 3 – Future Intersection LOS and Queue Summary w/Project

Int.	Lane	Storage	Year 2027 w/Project						Year 2044 w/Project					
			AM Peak			PM Peak			AM Peak			PM Peak		
			Delay (s)	LOS	Q ⁽¹⁾	Delay (s)	LOS	Q ⁽¹⁾	Delay (s)	LOS	Q ⁽¹⁾	Delay (s)	LOS	Q ⁽¹⁾
Chambers Rd & 104th Ave	Overall	-	45	D	-	39	D	-	33	C	-	32	C	-
	EB LT	275'	93	F	86'	60	E	169'	59	E	112'	56	E	211'
	EB TH+RT	-	10	B	462'	16	B	308'	6	A	161'	5	A	287'
	EB RT	-	-	-	-	-	-	-	6	A	19'	6	A	25'
	WB LT	380'	68	E	71'	75	E	91'	63	E	92'	61	E	106'
	WB TH	-	33	C	511'	38	D	394'	31	C	498'	32	C	412'
	WB RT	350'	0	A	38'	0	A	23'	0	A	61'	0	A	91'
	NB LT	290'	>120	F	429'	46	D	329'	37	D	176'	37	D	157'
	NB TH+RT	-	41	D	122'	43	D	213'	45	D	153'	53	D	234'
	NB RT	-	-	-	-	-	-	-	49	D	80'	55	E	69'
	SB LT	285'	39	D	105'	41	D	140'	41	D	141'	42	D	189'
SB TH+RT	-	47	D	171'	71	E	232'	47	D	139'	49	D	239'	
SB RT	-	-	-	-	-	-	-	95	F	223'	32	C	67'	
Chambers Rd & 96th Ave	Overall	-	32	C	-	33	C	-	25	C	-	31	C	-
	EB LT	70'	38	D	93'	39	D	327'	32	C	104'	76	E	462'
	EB TH+RT	-	9	A	134'	10	B	463'	9	A	88'	9	A	256'
	WB LT+TH	-	42	D	643'	67	E	599'	27	C	394'	34	C	353'
	WB RT	-	15	B	46'	19	B	51'	18	B	51'	26	C	58'
	NB LT+TH+RT	-	58	E	0'	63	E	6'	49	D	0'	53	D	8'
	SB LT	85'	29	C	190'	31	C	172'	24	C	255'	40	D	266'
	SB TH+RT	-	46	D	44'	28	C	42'	37	D	66'	28	C	50'
Chambers Rd & 100th Ave	Overall	-	10	A	-	12	B	-	19	B	-	19	B	-
	EB LT	150'	70	E	80'	>120	F	100'	56	E	161'	87	F	130'
	EB TH+RT	-	14	B	10'	18	B	5'	38	D	46'	44	D	35'
	WB LT	100'	39	D	43'	69	E	40'	43	D	116'	47	D	73'
	WB TH+RT	-	13	B	28'	17	B	38'	46	D	76'	81	F	80'
	NB LT	235'	8	A	0'	8	A	3'	10	A	10'	6	A	20'
	NB TH+RT	-	0	A	0'	0	A	0'	8	A	203'	6	A	251'
	SB LT	235'	8	A	10'	9	A	18'	15	B	57'	18	B	125'
	SB TH	-	0	A	0'	0	A	0'	8	A	116'	5	A	73'
SB RT	135'	0	A	0'	0	A	0'	6	A	4'	3	A	1'	
Sable Blvd & 104th Ave	Overall	-	17	B	-	7	A	-	13	B	-	16	B	-
	EB TH+RT	-	7	A	314'	7	A	425'	9	A	326'	7	A	389'
	WB LT	300'	20	B	14'	11	B	99'	19	B	86'	140	F	289'
	WB TH	-	17	B	511'	0	A	561'	9	A	674'	13	B	476'
	NB LT	125'	59	E	221'	62	E	134'	57	E	267'	61	E	151'
NB RT	-	49	D	54'	57	E	70'	46	D	118'	58	E	113'	
Sable Blvd & 102nd Ave	Overall	-	5	A	-	5	A	-	6	A	-	5	A	-
	EB LT+TH+RT	-	0	A	0'	0	A	0'	0	A	0'	0	A	0'
	WB LT+TH+RT	-	9	A	5'	9	A	3'	9	A	5'	9	A	3'
	NB LT+TH+RT	-	0	A	0'	0	A	0'	0	A	0'	0	A	0'
	SB LT+TH+RT	-	7	A	0'	7	A	3'	7	A	0'	7	A	3'
Chambers Road & 102nd Ave	Overall	-	2	A	-	1	A	-	3	A	-	2	A	-
	EB LT	-	26	D	25'	35	D	20'	52	F	63'	67	F	48'
	EB RT	50'	12	B	5'	13	B	3'	13	B	5'	15	B	3'
	NB LT	175'	8	A	0'	9	A	3'	9	A	0'	10	A	3'
	NB TH	-	0	A	0'	0	A	0'	0	A	0'	0	A	0'
	SB TH	-	0	A	0'	0	A	0'	0	A	0'	0	A	0'
	SB TH+RT	135'	0	A	0'	0	A	0'	0	A	0'	0	A	0'
E. Access & 102nd	Overall	-	5	A	-	3	A	-	4	A	-	2	A	-
	EB LT+TH	-	7	A	0'	7	A	0'	7	A	0'	7	A	0'
	WB TH+RT	-	0	A	0'	0	A	0'	0	A	0'	0	A	0'
	SB LT+RT	-	9	A	5'	9	A	3'	9	A	5'	9	A	3'
W. Access/Altura & 102nd	Overall	-	6	A	-	3	A	-	6	A	-	4	A	-
	EB LT+TH+RT	-	7	A	0'	7	A	0'	7	A	0'	7	A	0'
	WB LT+TH+RT	-	0	A	0'	0	A	0'	7	A	0'	7	A	0'
	NB LT+TH+RT	-	0	A	0'	0	A	0'	9	A	0'	9	A	0'
	SB LT+TH+RT	-	9	A	3'	9	A	3'	9	A	3'	9	A	3'

⁽¹⁾ 95th-percentile queue length as calculated by Synchro/HCM
 Note: Delay represented in average seconds per vehicle.

6.3 Site Access and Circulation

The project proposes the following access points and configurations:

- Full-movement access along E. 102nd Avenue approximately 370' west of Chambers Road
- Full-movement access aligning with Altura Street approximately 1,200' west of Chambers Road

The project proposes to extend E. 102nd Avenue from Chambers to the west along the project frontage consistent with the City's "Minor/Residential Collector" standards per the C3 Vision Transportation Plan Roadway Classification Plan. This includes construction of the E. 102nd Avenue & Chambers Road intersection as a full-movement, side-street stop-sign controlled intersection, which will provide access to the project via Chambers Road. Per the concept site plan, a detached sidewalk is proposed along the north side of E. 102nd Avenue along the project frontage. Internal drive aisles and interior parking throughout the site allows direct access to each of the proposed buildings and access to on-site amenities without traveling on exterior roadways.

As noted in Section 6.0, with the connectivity of E. 102nd Avenue to Chambers Road, it is anticipated that some existing traffic from the western portion of the Harvest Meadows neighborhood to the south, as well as future trips from the Anderson Ranch development to the west (on the order of 5-10 vehicles per peak hour), may shift to this connection as it will provide more direct access to Chambers Road and the E. 104th Avenue corridor. This will result in a change in traffic patterns within the adjacent Harvest Meadows neighborhood, with a likely reduced use of E. 101st Avenue for access to/from the western portion of Harvest Meadows to access the E. 104th Avenue corridor, in favor of increased use of E. 102nd Avenue.

Based on the projected site trips at buildout and accounting for this shift of some existing/future traffic to the E. 102nd Avenue & Chambers Road intersection, the following auxiliary lanes are prescribed per Section 3.04 of the City of Commerce City Engineering Construction Standards and Specifications:

- Construct a northbound left-turn deceleration lane = 175' lane + 180' (15:1) taper; this includes a 40' storage component as required by City standards
- Construct a southbound right-turn deceleration lane = 135' + 180' (15:1) taper

6.4 Traffic Signal Warrants

An analysis of a potential traffic signal at the E. 102nd Avenue & Chambers Road intersection was performed consistent with Manual on Uniform Traffic Control Devices (MUTCD) criteria.

The MUTCD provides criteria for nine different warrants. For the purposes of this planning-level analysis, MUTCD “Peak Hour”, warrant criteria was applied and used as an indicator to determine applicability of a signal at this location with 20-year buildout traffic volumes. These warrants consider the hourly volume of both the “major street” (Chambers Road) and the “minor street” approaches (E. 102nd Avenue eastbound approach).

The MUTCD Peak Hour signal warrant analysis graph with plots of the projected Year 2044 AM and PM peak hour volume totals is provided in the Appendix. Consistent with MUTCD criteria, eastbound right-turns from E. 102nd Avenue were not included since the LOS and queue analysis shows that these turns can be made onto Chambers Road with minimal conflict. Thus, only the left-turn volumes were applied, and the approach was evaluated as a one-lane approach on the warrant chart.

Based on the projected traffic volumes and MUTCD Peak Hour criteria, this intersection is not anticipated to meet any signal warrant thresholds in the 20-year buildout scenario.

7.0 CONCLUSIONS AND RECOMMENDATIONS

This traffic impact study has been prepared by the Fox Tuttle Transportation Group for the Chambers Road Apartments project in the City of Commerce City. The project proposes to develop multifamily rental residential units at the northwest quadrant of the E. 102nd Avenue and Chambers Road intersection.

The project proposes 283 apartments with access via two full-movement intersections along E. 102nd Avenue. The project is anticipated to generate approximately 1,907 daily, 113 AM peak hour and 144 PM peak hour driveway trips at full buildout and occupancy. **It was determined that the existing and future roadway and intersection network can serve the project at acceptable Levels of Service per City criteria in the short and long-term with the following background (non-project related) and project-related improvements in place:**

Long-Term (Year 2044) Background (non-project related) Improvement Recommendations:

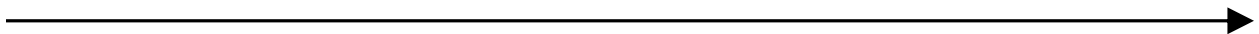
- If future regional growth projections are realized, the intersection of E. 104th Avenue & Chambers Blvd. will need to be constructed to its buildout lane geometry, with three (3) through lanes in each direction eastbound and westbound on E. 104th Avenue and exclusive right-turn lanes added on the eastbound, northbound and southbound approaches.
- If future regional growth projections are realized, the intersection of E. 96th Avenue & Chambers Blvd. will need to be constructed to its buildout lane geometry, with two (2) through lanes in each direction eastbound and westbound on E. 96th Avenue.
- Roadway improvements identified by the adjacent Anderson Ranch project were assumed to be implemented in the background for this study. Those improvements include:
 - Install traffic signal control at E. 104th Avenue and Sable Boulevard. Extend northbound left turn storage at the intersection to 125 feet.
 - Add eastbound, northbound, and southbound left turn auxiliary lanes and southbound right turn auxiliary lane at E. 100th Avenue and Chambers Road. Install traffic signal control at the intersection.
 - Extend eastbound left turn storage at E. 96th Avenue & Chambers Road to 425 feet. Extend southbound right turn storage at E. 96th Avenue & Chambers Road to 175 feet.

Project-Related Improvement Recommendations (Responsibility of the Developer):

- Extend E. 102nd Avenue from Chambers to the west along the project frontage. This roadway should be built consistent with the City's "Minor/Residential Collector" standards per the C3 Vision Transportation Plan Roadway Classification Plan.
- The intersection of E. 102nd Avenue should be built as a side-street, stop-controlled intersection to include a northbound left-turn deceleration lane (175' lane + 180' taper) and a southbound right-turn deceleration lane (135' + 180'), consistent with the City of Commerce City Engineering Construction Standards and Specifications. The eastbound approach should be striped with a 50' left turn lane.
- Construct the east access to the site along E. 102nd Avenue with side-street stop-sign control on the southbound approach. No auxiliary turn lanes are required or recommended at this access given the Minor Collector roadway classification, consistent with City standards.
- Construct the west access to the site along E. 102nd Avenue to align with Altura Street, with side-street stop-sign control on the northbound and southbound approaches. No auxiliary turn lanes are required or recommended at this access given the Minor Collector roadway classification, consistent with City standards.

APPENDIX

Level of Service Definitions
Intersection Capacity Worksheets
MUTCD Peak Hour Signal Warrant
Traffic Count Data Sheets



Level of Service Definitions



LEVEL OF SERVICE (LOS) DEFINITIONS

In rating roadway and intersection operating conditions with existing or future traffic volumes, “Levels of Service” (LOS) A through F are used, with LOS A indicating very good operation and LOS F indicating poor operation. Levels of service at signalized and unsignalized intersections are closely associated with vehicle delays experienced in seconds per vehicle. More complete level of service definitions and delay data for signal and stop sign controlled intersections are contained in the following table for reference.

Level of Service Rating	Delay in seconds per vehicle*		Definition
	Signalized	Unsignalized	
A	0.0 to 10.0	0.0 to 10.0	Low vehicular traffic volumes; primarily free flow operations. Density is low and vehicles can freely maneuver within the traffic stream. Drivers can maintain their desired speeds with little or no delay.
B	10.1 to 20.0	10.1 to 15.0	Stable vehicular traffic volume flow with potential for some restriction of operating speeds due to traffic conditions. Vehicle maneuvering is only slightly restricted. The stopped delays are not bothersome, and drivers are not subject to appreciable tension.
C	20.1 to 35.0	15.1 to 25.0	Stable traffic operations, however, the ability for vehicles to maneuver is more restricted by the increase in traffic volumes. Relatively satisfactory operating speeds prevail, but adverse signal coordination or longer vehicle queues cause delays along the corridor.
D	35.1 to 55.0	25.1 to 35.0	Approaching unstable vehicular traffic flow where small increases in volume could cause substantial delays. Most drivers are restricted in ability to maneuver and selection of travel speeds due to congestion. Driver comfort and convenience are low, but tolerable.
E	55.1 to 80.0	35.1 to 50.0	Traffic operations characterized by significant approach delays and average travel speeds of one-half to one-third the free flow speed. Vehicular flow is unstable and there is potential for stoppages of brief duration. High signal density, extensive vehicle queuing, or corridor signal progression/timing are the typical causes of vehicle delays at signalized corridors.
F	> 80.0	> 50.0	Forced vehicular traffic flow and operations with high approach delays at critical intersections. Vehicle speeds are reduced substantially and stoppages may occur for short or long periods of time because of downstream congestion.

* Delay ranges based on 2010 Highway Capacity Manual Criteria

Intersection Capacity Worksheets



Timings
1: Chambers Rd & E. 104th Ave

Existing AM
Chambers Rd Apartments Traffic Impact Study

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	103	715	74	952	190	216	168	223	147
Future Volume (vph)	103	715	74	952	190	216	168	223	147
Turn Type	Prot	NA	Prot	NA	Perm	pm+pt	NA	pm+pt	NA
Protected Phases	5	2	1	6		7	4	3	8
Permitted Phases					6	4		8	
Detector Phase	5	2	1	6	6	7	4	3	8
Switch Phase									
Minimum Initial (s)	3.0	15.0	3.0	15.0	15.0	3.0	5.0	3.0	5.0
Minimum Split (s)	12.0	37.0	12.0	37.0	37.0	11.7	34.7	11.7	34.7
Total Split (s)	22.0	44.0	22.0	44.0	44.0	19.0	35.0	19.0	35.0
Total Split (%)	18.3%	36.7%	18.3%	36.7%	36.7%	15.8%	29.2%	15.8%	29.2%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	4.7	4.7	4.7	4.7
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0	6.7	6.7	6.7	6.7
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Min	None	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	9.4	59.3	8.5	58.4	58.4	25.6	13.3	24.0	12.5
Actuated g/C Ratio	0.08	0.49	0.07	0.49	0.49	0.21	0.11	0.20	0.10
v/c Ratio	0.43	0.56	0.37	0.67	0.26	1.01	0.66	0.48	0.72
Control Delay	57.4	23.2	57.0	26.9	3.4	99.9	47.2	37.9	29.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	57.4	23.2	57.0	26.9	3.4	99.9	47.2	37.9	29.0
LOS	E	C	E	C	A	F	D	D	C
Approach Delay		27.0		25.0			71.8		32.5
Approach LOS		C		C			E		C

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 8 (7%), Referenced to phase 2:EBT and 6:WBT, Start of Yellow
 Natural Cycle: 100
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.01
 Intersection Signal Delay: 33.5
 Intersection LOS: C
 Intersection Capacity Utilization 75.0%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 1: Chambers Rd & E. 104th Ave



Queues
1: Chambers Rd & E. 104th Ave

Existing AM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	117	955	89	1147	229	245	280	245	385
v/c Ratio	0.43	0.56	0.37	0.67	0.26	1.01	0.66	0.48	0.72
Control Delay	57.4	23.2	57.0	26.9	3.4	99.9	47.2	37.9	29.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	57.4	23.2	57.0	26.9	3.4	99.9	47.2	37.9	29.0
Queue Length 50th (ft)	45	262	34	350	0	~163	86	77	62
Queue Length 95th (ft)	72	351	55	422	34	#268	125	107	112
Internal Link Dist (ft)		3830		955			548		948
Turn Bay Length (ft)	275		380		350	290		285	
Base Capacity (vph)	429	1719	429	1721	887	243	840	534	932
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.27	0.56	0.21	0.67	0.26	1.01	0.33	0.46	0.41

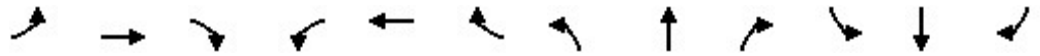
Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
 1: Chambers Rd & E. 104th Ave

Existing AM
 Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔↔	↕↔		↔↔	↕↕	↔	↔	↕↔		↔↔	↕↔	
Traffic Volume (veh/h)	103	715	125	74	952	190	216	168	78	223	147	203
Future Volume (veh/h)	103	715	125	74	952	190	216	168	78	223	147	203
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	117	812	142	89	1147	0	245	191	89	245	162	223
Peak Hour Factor	0.88	0.88	0.88	0.83	0.83	0.83	0.88	0.88	0.88	0.91	0.91	0.91
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	174	1398	244	141	1609		272	458	205	637	295	263
Arrive On Green	0.05	0.46	0.46	0.04	0.45	0.00	0.10	0.19	0.19	0.08	0.17	0.17
Sat Flow, veh/h	3456	3023	529	3456	3554	1585	1781	2386	1069	3456	1777	1585
Grp Volume(v), veh/h	117	477	477	89	1147	0	245	140	140	245	162	223
Grp Sat Flow(s),veh/h/ln	1728	1777	1775	1728	1777	1585	1781	1777	1678	1728	1777	1585
Q Serve(g_s), s	4.0	23.7	23.7	3.0	31.3	0.0	12.3	8.3	8.8	6.9	10.0	16.4
Cycle Q Clear(g_c), s	4.0	23.7	23.7	3.0	31.3	0.0	12.3	8.3	8.8	6.9	10.0	16.4
Prop In Lane	1.00		0.30	1.00		1.00	1.00		0.64	1.00		1.00
Lane Grp Cap(c), veh/h	174	821	821	141	1609		272	341	322	637	295	263
V/C Ratio(X)	0.67	0.58	0.58	0.63	0.71		0.90	0.41	0.43	0.38	0.55	0.85
Avail Cap(c_a), veh/h	432	821	821	432	1609		272	419	396	727	419	374
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	56.0	23.7	23.7	56.6	26.5	0.0	40.4	42.5	42.7	37.2	45.9	48.6
Incr Delay (d2), s/veh	4.4	3.0	3.0	4.5	2.7	0.0	30.3	0.8	0.9	0.4	1.6	11.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.8	10.1	10.0	1.4	13.0	0.0	8.2	3.7	3.7	2.9	4.5	7.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	60.4	26.7	26.7	61.2	29.3	0.0	70.6	43.3	43.7	37.6	47.5	60.5
LnGrp LOS	E	C	C	E	C		E	D	D	D	D	E
Approach Vol, veh/h		1071			1236			525			630	
Approach Delay, s/veh		30.4			31.6			56.2			48.2	
Approach LOS		C			C			E			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.9	62.5	15.9	29.7	13.1	61.3	19.0	26.6				
Change Period (Y+Rc), s	7.0	7.0	6.7	6.7	7.0	7.0	6.7	6.7				
Max Green Setting (Gmax), s	15.0	37.0	12.3	28.3	15.0	37.0	12.3	28.3				
Max Q Clear Time (g_c+I1), s	5.0	25.7	8.9	10.8	6.0	33.3	14.3	18.4				
Green Ext Time (p_c), s	0.1	4.3	0.3	1.3	0.2	2.3	0.0	1.5				

Intersection Summary

HCM 6th Ctrl Delay	38.0
HCM 6th LOS	D

Notes

- User approved pedestrian interval to be less than phase max green.
- Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

Timings
2: Chambers Rd & 96th Ave

Existing AM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	54	265	577	131	0	157	2
Future Volume (vph)	54	265	577	131	0	157	2
Turn Type	Prot	NA	NA	Perm	NA	Split	NA
Protected Phases	5	2	6		8	4	4
Permitted Phases				6			
Detector Phase	5	2	6	6	8	4	4
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Total Split (s)	23.0	59.0	36.0	36.0	23.0	23.0	23.0
Total Split (%)	21.9%	56.2%	34.3%	34.3%	21.9%	21.9%	21.9%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?	Yes		Yes	Yes			
Recall Mode	None	Min	None	None	None	None	None
Act Effect Green (s)	8.1	40.6	32.9	32.9	5.7	12.6	12.6
Actuated g/C Ratio	0.12	0.62	0.51	0.51	0.09	0.19	0.19
v/c Ratio	0.30	0.28	0.71	0.18	0.02	0.57	0.41
Control Delay	33.2	7.4	22.9	5.4	0.0	32.9	7.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.2	7.4	22.9	5.4	0.0	32.9	7.9
LOS	C	A	C	A	A	C	A
Approach Delay		11.8	19.7				20.6
Approach LOS		B	B				C

Intersection Summary

Cycle Length: 105
 Actuated Cycle Length: 65.1
 Natural Cycle: 105
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 0.71
 Intersection Signal Delay: 17.9
 Intersection Capacity Utilization 68.6%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service C

Splits and Phases: 2: Chambers Rd & 96th Ave



Queues
2: Chambers Rd & 96th Ave

Existing AM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	66	324	671	152	8	194	188
v/c Ratio	0.30	0.28	0.71	0.18	0.02	0.57	0.41
Control Delay	33.2	7.4	22.9	5.4	0.0	32.9	7.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.2	7.4	22.9	5.4	0.0	32.9	7.9
Queue Length 50th (ft)	25	45	215	6	0	73	1
Queue Length 95th (ft)	64	124	#544	43	0	141	39
Internal Link Dist (ft)		1966	2271		176		2115
Turn Bay Length (ft)	70						
Base Capacity (vph)	507	1571	941	862	763	507	587
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.13	0.21	0.71	0.18	0.01	0.38	0.32

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary

2: Chambers Rd & 96th Ave

Existing AM
Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	54	265	1	0	577	131	0	0	3	157	2	151
Future Volume (veh/h)	54	265	1	0	577	131	0	0	3	157	2	151
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	66	323	1	0	671	152	0	0	8	194	2	186
Peak Hour Factor	0.82	0.82	0.82	0.86	0.86	0.86	0.38	0.38	0.38	0.81	0.81	0.81
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	146	1081	3	0	779	660	0	0	16	291	3	257
Arrive On Green	0.08	0.58	0.58	0.00	0.42	0.42	0.00	0.00	0.01	0.16	0.16	0.16
Sat Flow, veh/h	1781	1864	6	0	1870	1585	0	0	1585	1781	17	1571
Grp Volume(v), veh/h	66	0	324	0	671	152	0	0	8	194	0	188
Grp Sat Flow(s),veh/h/ln	1781	0	1869	0	1870	1585	0	0	1585	1781	0	1588
Q Serve(g_s), s	2.2	0.0	5.4	0.0	19.9	3.8	0.0	0.0	0.3	6.2	0.0	6.9
Cycle Q Clear(g_c), s	2.2	0.0	5.4	0.0	19.9	3.8	0.0	0.0	0.3	6.2	0.0	6.9
Prop In Lane	1.00		0.00	0.00		1.00	0.00		1.00	1.00		0.99
Lane Grp Cap(c), veh/h	146	0	1085	0	779	660	0	0	16	291	0	259
V/C Ratio(X)	0.45	0.00	0.30	0.00	0.86	0.23	0.00	0.00	0.49	0.67	0.00	0.72
Avail Cap(c_a), veh/h	526	0	1656	0	951	806	0	0	468	526	0	469
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	26.7	0.0	6.5	0.0	16.2	11.5	0.0	0.0	30.0	23.9	0.0	24.2
Incr Delay (d2), s/veh	2.2	0.0	0.2	0.0	7.0	0.2	0.0	0.0	20.5	2.6	0.0	3.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0	0.0	1.7	0.0	8.8	1.2	0.0	0.0	0.2	2.6	0.0	2.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	28.9	0.0	6.7	0.0	23.2	11.7	0.0	0.0	50.5	26.6	0.0	28.0
LnGrp LOS	C	A	A	A	C	B	A	A	D	C	A	C
Approach Vol, veh/h		390			823			8				382
Approach Delay, s/veh		10.4			21.0			50.5				27.3
Approach LOS		B			C			D				C
Timer - Assigned Phs		2		4	5	6		8				
Phs Duration (G+Y+Rc), s		40.4		15.0	10.0	30.4		5.6				
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		5.0				
Max Green Setting (Gmax), s		54.0		18.0	18.0	31.0		18.0				
Max Q Clear Time (g_c+I1), s		7.4		8.9	4.2	21.9		2.3				
Green Ext Time (p_c), s		2.2		1.1	0.1	3.5		0.0				
Intersection Summary												
HCM 6th Ctrl Delay				20.1								
HCM 6th LOS				C								

Intersection												
Int Delay, s/veh	5.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕		↕	↕			↕			↕	
Traffic Vol, veh/h	6	10	17	57	8	133	3	252	32	116	262	4
Future Vol, veh/h	6	10	17	57	8	133	3	252	32	116	262	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	100	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	7	11	18	62	9	145	3	274	35	126	285	4

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	914	854	287	852	839	292	289	0	0	309	0	0
Stage 1	539	539	-	298	298	-	-	-	-	-	-	-
Stage 2	375	315	-	554	541	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	254	296	752	280	302	747	1273	-	-	1252	-	-
Stage 1	527	522	-	711	667	-	-	-	-	-	-	-
Stage 2	646	656	-	517	521	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	181	260	752	240	265	747	1273	-	-	1252	-	-
Mov Cap-2 Maneuver	181	260	-	240	265	-	-	-	-	-	-	-
Stage 1	525	459	-	709	665	-	-	-	-	-	-	-
Stage 2	513	654	-	433	458	-	-	-	-	-	-	-

Approach	EB		WB		NB			SB		
HCM Control Delay, s	16.5		15.7		0.1			2.5		
HCM LOS	C		C							

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1273	-	-	350	240	677	1252	-	-
HCM Lane V/C Ratio	0.003	-	-	0.102	0.258	0.226	0.101	-	-
HCM Control Delay (s)	7.8	0	-	16.5	25.1	11.9	8.2	0	-
HCM Lane LOS	A	A	-	C	D	B	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0.3	1	0.9	0.3	-	-

Intersection						
Int Delay, s/veh	29.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↖	↑↑	↖	↖
Traffic Vol, veh/h	1065	6	38	1276	105	63
Future Vol, veh/h	1065	6	38	1276	105	63
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	300	-	70	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1158	7	41	1387	114	68

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	1165	0	1938 583
Stage 1	-	-	-	-	1162 -
Stage 2	-	-	-	-	776 -
Critical Hdwy	-	-	4.14	-	6.84 6.94
Critical Hdwy Stg 1	-	-	-	-	5.84 -
Critical Hdwy Stg 2	-	-	-	-	5.84 -
Follow-up Hdwy	-	-	2.22	-	3.52 3.32
Pot Cap-1 Maneuver	-	-	595	-	~ 57 456
Stage 1	-	-	-	-	260 -
Stage 2	-	-	-	-	414 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	595	-	~ 53 456
Mov Cap-2 Maneuver	-	-	-	-	~ 53 -
Stage 1	-	-	-	-	260 -
Stage 2	-	-	-	-	385 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0.3	\$ 441.2
HCM LOS			F

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	53	456	-	-	595	-
HCM Lane V/C Ratio	2.153	0.15	-	-	0.069	-
HCM Control Delay (s)	\$ 697.4	14.3	-	-	11.5	-
HCM Lane LOS	F	B	-	-	B	-
HCM 95th %tile Q(veh)	11.4	0.5	-	-	0.2	-

Notes
 ~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Timings
1: Chambers Rd & E. 104th Ave

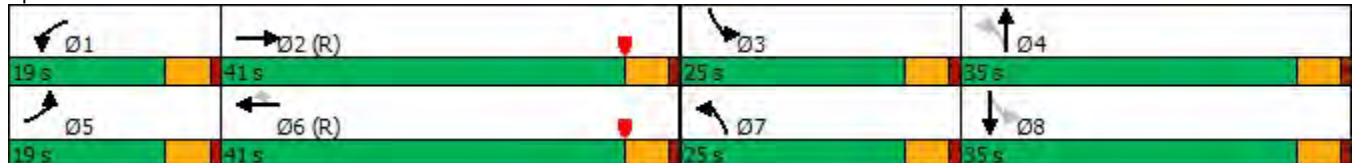
Existing PM
Chambers Rd Apartments Traffic Impact Study

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	251	919	119	924	291	274	315	352	321
Future Volume (vph)	251	919	119	924	291	274	315	352	321
Turn Type	Prot	NA	Prot	NA	Perm	pm+pt	NA	pm+pt	NA
Protected Phases	5	2	1	6		7	4	3	8
Permitted Phases					6	4		8	
Detector Phase	5	2	1	6	6	7	4	3	8
Switch Phase									
Minimum Initial (s)	10.0	15.0	10.0	15.0	15.0	10.0	15.0	10.0	15.0
Minimum Split (s)	15.0	24.0	15.0	24.5	24.5	15.0	24.0	15.0	24.5
Total Split (s)	19.0	41.0	19.0	41.0	41.0	25.0	35.0	25.0	35.0
Total Split (%)	15.8%	34.2%	15.8%	34.2%	34.2%	20.8%	29.2%	20.8%	29.2%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Min	None	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	14.8	48.2	11.3	44.7	44.7	44.0	25.1	37.1	21.6
Actuated g/C Ratio	0.12	0.40	0.09	0.37	0.37	0.37	0.21	0.31	0.18
v/c Ratio	0.68	0.94	0.48	0.91	0.46	0.86	0.66	0.63	0.76
Control Delay	58.9	48.7	56.5	48.3	5.1	52.4	43.1	30.5	50.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	58.9	48.7	56.5	48.3	5.1	52.4	43.1	30.5	50.8
LOS	E	D	E	D	A	D	D	C	D
Approach Delay		50.5		39.6			46.6		41.6
Approach LOS		D		D			D		D

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 1 (1%), Referenced to phase 2:EBT and 6:WBT, Start of Yellow
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.94
 Intersection Signal Delay: 44.6
 Intersection LOS: D
 Intersection Capacity Utilization 85.2%
 ICU Level of Service E
 Analysis Period (min) 15

Splits and Phases: 1: Chambers Rd & E. 104th Ave



Queues
1: Chambers Rd & E. 104th Ave

Existing PM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	289	1311	155	1200	378	304	497	400	489
v/c Ratio	0.68	0.94	0.48	0.91	0.46	0.86	0.66	0.63	0.76
Control Delay	58.9	48.7	56.5	48.3	5.1	52.4	43.1	30.5	50.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	58.9	48.7	56.5	48.3	5.1	52.4	43.1	30.5	50.8
Queue Length 50th (ft)	111	501	59	470	0	172	167	109	177
Queue Length 95th (ft)	151	#726	78	#537	27	#289	217	131	217
Internal Link Dist (ft)		3830		955			548		948
Turn Bay Length (ft)	275		380		350	290		285	
Base Capacity (vph)	440	1394	400	1317	826	368	884	770	879
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.66	0.94	0.39	0.91	0.46	0.83	0.56	0.52	0.56

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary

Existing PM

1: Chambers Rd & E. 104th Ave

Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	251	919	222	119	924	291	274	315	132	352	321	109
Future Volume (veh/h)	251	919	222	119	924	291	274	315	132	352	321	109
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	289	1056	255	155	1200	0	304	350	147	400	365	124
Peak Hour Factor	0.87	0.87	0.87	0.77	0.77	0.77	0.90	0.90	0.90	0.88	0.88	0.88
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	347	1208	290	286	1448		366	512	211	653	447	150
Arrive On Green	0.10	0.43	0.43	0.08	0.41	0.00	0.15	0.21	0.21	0.12	0.17	0.17
Sat Flow, veh/h	3456	2842	683	3456	3554	1585	1781	2452	1013	3456	2614	875
Grp Volume(v), veh/h	289	658	653	155	1200	0	304	252	245	400	246	243
Grp Sat Flow(s),veh/h/ln	1728	1777	1747	1728	1777	1585	1781	1777	1688	1728	1777	1713
Q Serve(g_s), s	9.9	40.6	41.1	5.2	36.3	0.0	16.3	15.7	16.1	11.2	16.0	16.4
Cycle Q Clear(g_c), s	9.9	40.6	41.1	5.2	36.3	0.0	16.3	15.7	16.1	11.2	16.0	16.4
Prop In Lane	1.00		0.39	1.00		1.00	1.00		0.60	1.00		0.51
Lane Grp Cap(c), veh/h	347	755	743	286	1448		366	371	352	653	304	293
V/C Ratio(X)	0.83	0.87	0.88	0.54	0.83		0.83	0.68	0.70	0.61	0.81	0.83
Avail Cap(c_a), veh/h	403	755	743	403	1448		388	444	422	826	444	428
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.0	31.5	31.7	52.8	31.8	0.0	33.5	43.8	44.0	35.1	47.9	48.0
Incr Delay (d2), s/veh	12.3	13.2	14.0	1.6	5.6	0.0	13.5	3.2	3.9	0.9	7.1	8.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.8	19.1	19.1	2.3	15.8	0.0	8.2	7.1	7.0	4.7	7.5	7.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	65.2	44.7	45.6	54.4	37.4	0.0	47.1	47.0	47.8	36.1	54.9	56.5
LnGrp LOS	E	D	D	D	D		D	D	D	D	D	E
Approach Vol, veh/h		1600			1355			801			889	
Approach Delay, s/veh		48.8			39.4			47.3			46.9	
Approach LOS		D			D			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	14.9	56.0	19.0	30.0	17.1	53.9	23.5	25.5				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	14.0	36.0	20.0	30.0	14.0	36.0	20.0	30.0				
Max Q Clear Time (g_c+I1), s	7.2	43.1	13.2	18.1	11.9	38.3	18.3	18.4				
Green Ext Time (p_c), s	0.2	0.0	0.8	2.2	0.2	0.0	0.2	2.1				

Intersection Summary

HCM 6th Ctrl Delay	45.4
HCM 6th LOS	D

Notes

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

Timings
2: Chambers Rd & 96th Ave

Existing PM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	249	788	598	167	1	182	1
Future Volume (vph)	249	788	598	167	1	182	1
Turn Type	Prot	NA	NA	Perm	NA	Split	NA
Protected Phases	5	2	6		8	4	4
Permitted Phases				6			
Detector Phase	5	2	6	6	8	4	4
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Total Split (s)	23.0	49.0	26.0	26.0	23.0	23.0	23.0
Total Split (%)	24.2%	51.6%	27.4%	27.4%	24.2%	24.2%	24.2%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?	Yes		Yes	Yes			
Recall Mode	None	Min	None	None	None	None	None
Act Effct Green (s)	14.9	41.4	21.4	21.4	5.8	12.9	12.9
Actuated g/C Ratio	0.22	0.62	0.32	0.32	0.09	0.19	0.19
v/c Ratio	0.65	0.70	1.04	0.29	0.03	0.61	0.22
Control Delay	33.3	14.4	76.4	7.4	28.0	33.8	8.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.3	14.4	76.4	7.4	28.0	33.8	8.5
LOS	C	B	E	A	C	C	A
Approach Delay		19.0	61.3		28.0		26.6
Approach LOS		B	E		C		C

Intersection Summary

Cycle Length: 95	
Actuated Cycle Length: 66.3	
Natural Cycle: 105	
Control Type: Semi Act-Uncoord	
Maximum v/c Ratio: 1.04	
Intersection Signal Delay: 35.7	Intersection LOS: D
Intersection Capacity Utilization 102.2%	ICU Level of Service G
Analysis Period (min) 15	

Splits and Phases: 2: Chambers Rd & 96th Ave



Queues
2: Chambers Rd & 96th Ave



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	257	812	629	176	4	209	83
v/c Ratio	0.65	0.70	1.04	0.29	0.03	0.61	0.22
Control Delay	33.3	14.4	76.4	7.4	28.0	33.8	8.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.3	14.4	76.4	7.4	28.0	33.8	8.5
Queue Length 50th (ft)	90	170	~274	7	1	75	0
Queue Length 95th (ft)	#207	#536	#634	59	6	160	33
Internal Link Dist (ft)		1966	2271		176		2115
Turn Bay Length (ft)	70						
Base Capacity (vph)	490	1262	602	613	482	490	499
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.52	0.64	1.04	0.29	0.01	0.43	0.17

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary

2: Chambers Rd & 96th Ave

Existing PM
Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	249	788	0	0	598	167	0	1	1	182	1	71
Future Volume (veh/h)	249	788	0	0	598	167	0	1	1	182	1	71
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	257	812	0	0	629	176	0	2	2	209	1	82
Peak Hour Factor	0.97	0.97	0.97	0.95	0.95	0.95	0.50	0.50	0.50	0.87	0.87	0.87
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	317	1118	0	0	634	537	0	5	5	276	3	243
Arrive On Green	0.18	0.60	0.00	0.00	0.34	0.34	0.00	0.01	0.01	0.15	0.15	0.15
Sat Flow, veh/h	1781	1870	0	0	1870	1585	0	858	858	1781	19	1569
Grp Volume(v), veh/h	257	812	0	0	629	176	0	0	4	209	0	83
Grp Sat Flow(s),veh/h/ln	1781	1870	0	0	1870	1585	0	0	1716	1781	0	1588
Q Serve(g_s), s	8.6	19.1	0.0	0.0	20.8	5.1	0.0	0.0	0.1	7.0	0.0	2.9
Cycle Q Clear(g_c), s	8.6	19.1	0.0	0.0	20.8	5.1	0.0	0.0	0.1	7.0	0.0	2.9
Prop In Lane	1.00		0.00	0.00		1.00	0.00		0.50	1.00		0.99
Lane Grp Cap(c), veh/h	317	1118	0	0	634	537	0	0	9	276	0	246
V/C Ratio(X)	0.81	0.73	0.00	0.00	0.99	0.33	0.00	0.00	0.43	0.76	0.00	0.34
Avail Cap(c_a), veh/h	518	1328	0	0	634	537	0	0	499	518	0	461
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.5	8.9	0.0	0.0	20.4	15.2	0.0	0.0	30.7	25.1	0.0	23.3
Incr Delay (d2), s/veh	5.0	1.6	0.0	0.0	33.7	0.4	0.0	0.0	29.1	4.3	0.0	0.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.8	6.3	0.0	0.0	14.0	1.7	0.0	0.0	0.1	3.0	0.0	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	29.4	10.5	0.0	0.0	54.1	15.6	0.0	0.0	59.8	29.3	0.0	24.2
LnGrp LOS	C	B	A	A	D	B	A	A	E	C	A	C
Approach Vol, veh/h		1069			805			4				292
Approach Delay, s/veh		15.1			45.6			59.8				27.9
Approach LOS		B			D			E				C
Timer - Assigned Phs		2		4	5	6		8				
Phs Duration (G+Y+Rc), s		42.0		14.6	16.0	26.0		5.3				
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		5.0				
Max Green Setting (Gmax), s		44.0		18.0	18.0	21.0		18.0				
Max Q Clear Time (g_c+I1), s		21.1		9.0	10.6	22.8		2.1				
Green Ext Time (p_c), s		6.4		0.7	0.4	0.0		0.0				
Intersection Summary												
HCM 6th Ctrl Delay					28.2							
HCM 6th LOS					C							

Intersection												
Int Delay, s/veh	4.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕		↕	↕			↕			↕	
Traffic Vol, veh/h	4	5	7	30	7	121	14	353	65	158	267	8
Future Vol, veh/h	4	5	7	30	7	121	14	353	65	158	267	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	100	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	4	5	8	33	8	132	15	384	71	172	290	9

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1159	1124	295	1095	1093	420	299	0	0	455	0	0
Stage 1	639	639	-	450	450	-	-	-	-	-	-	-
Stage 2	520	485	-	645	643	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	173	205	744	191	214	633	1262	-	-	1106	-	-
Stage 1	464	470	-	589	572	-	-	-	-	-	-	-
Stage 2	539	552	-	461	468	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	112	164	744	156	171	633	1262	-	-	1106	-	-
Mov Cap-2 Maneuver	112	164	-	156	171	-	-	-	-	-	-	-
Stage 1	457	382	-	580	563	-	-	-	-	-	-	-
Stage 2	414	543	-	366	380	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	23.5		17.6		0.3		3.2	
HCM LOS	C		C					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1262	-	-	212	156	552	1106	-	-
HCM Lane V/C Ratio	0.012	-	-	0.082	0.209	0.252	0.155	-	-
HCM Control Delay (s)	7.9	0	-	23.5	34.1	13.7	8.9	0	-
HCM Lane LOS	A	A	-	C	D	B	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0.3	0.8	1	0.5	-	-

Intersection						
Int Delay, s/veh	11.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↖	↑↑	↖	↖
Traffic Vol, veh/h	1374	19	77	1191	42	48
Future Vol, veh/h	1374	19	77	1191	42	48
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	300	-	70	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1493	21	84	1295	46	52

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	1514	0	2320 757
Stage 1	-	-	-	-	1504 -
Stage 2	-	-	-	-	816 -
Critical Hdwy	-	-	4.14	-	6.84 6.94
Critical Hdwy Stg 1	-	-	-	-	5.84 -
Critical Hdwy Stg 2	-	-	-	-	5.84 -
Follow-up Hdwy	-	-	2.22	-	3.52 3.32
Pot Cap-1 Maneuver	-	-	437	-	~ 32 350
Stage 1	-	-	-	-	170 -
Stage 2	-	-	-	-	395 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	437	-	~ 26 350
Mov Cap-2 Maneuver	-	-	-	-	~ 26 -
Stage 1	-	-	-	-	170 -
Stage 2	-	-	-	-	319 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0.9	\$ 329
HCM LOS			F

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	26	350	-	-	437	-
HCM Lane V/C Ratio	1.756	0.149	-	-	0.192	-
HCM Control Delay (s)	\$ 685.5	17.1	-	-	15.2	-
HCM Lane LOS	F	C	-	-	C	-
HCM 95th %tile Q(veh)	5.5	0.5	-	-	0.7	-

Notes
 ~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Timings
1: Chambers Rd & E. 104th Ave

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	112	790	94	1040	207	275	196	242	165
Future Volume (vph)	112	790	94	1040	207	275	196	242	165
Turn Type	Prot	NA	Prot	NA	Perm	pm+pt	NA	pm+pt	NA
Protected Phases	5	2	1	6		7	4	3	8
Permitted Phases					6	4		8	
Detector Phase	5	2	1	6	6	7	4	3	8
Switch Phase									
Minimum Initial (s)	3.0	15.0	3.0	15.0	15.0	3.0	5.0	3.0	5.0
Minimum Split (s)	10.5	37.0	10.5	37.0	37.0	10.5	34.7	10.5	34.7
Total Split (s)	22.0	44.0	22.0	44.0	44.0	19.0	35.0	19.0	35.0
Total Split (%)	18.3%	36.7%	18.3%	36.7%	36.7%	15.8%	29.2%	15.8%	29.2%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	4.7	4.7	4.7	4.7
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0	6.7	6.7	6.7	6.7
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Min	None	C-Min	C-Min	None	None	None	Max
Act Effct Green (s)	9.8	42.7	9.3	42.2	42.2	41.8	29.5	39.4	28.3
Actuated g/C Ratio	0.08	0.36	0.08	0.35	0.35	0.35	0.25	0.33	0.24
v/c Ratio	0.45	0.86	0.42	1.01	0.35	0.88	0.40	0.37	0.45
Control Delay	67.4	36.5	57.3	66.4	5.1	57.4	27.7	25.8	17.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	67.4	36.5	57.3	66.4	5.1	57.4	27.7	25.8	17.8
LOS	E	D	E	E	A	E	C	C	B
Approach Delay		39.8		56.3			41.4		20.8
Approach LOS		D		E			D		C

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 8 (7%), Referenced to phase 2:EBT and 6:WBT, Start of Yellow
 Natural Cycle: 105
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.01
 Intersection Signal Delay: 43.3
 Intersection Capacity Utilization 81.8%
 Analysis Period (min) 15
 Intersection LOS: D
 ICU Level of Service D

Splits and Phases: 1: Chambers Rd & E. 104th Ave



Queues
1: Chambers Rd & E. 104th Ave



Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	127	1068	113	1253	249	313	365	266	424
v/c Ratio	0.45	0.86	0.42	1.01	0.35	0.88	0.40	0.37	0.45
Control Delay	67.4	36.5	57.3	66.4	5.1	57.4	27.7	25.8	17.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	67.4	36.5	57.3	66.4	5.1	57.4	27.7	25.8	17.8
Queue Length 50th (ft)	54	392	43	~514	2	176	85	69	60
Queue Length 95th (ft)	85	#400	66	#613	43	#252	129	99	108
Internal Link Dist (ft)		3830		955			548		948
Turn Bay Length (ft)	275		380		350	290		285	
Base Capacity (vph)	429	1241	429	1244	716	354	904	760	948
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.30	0.86	0.26	1.01	0.35	0.88	0.40	0.35	0.45

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.





























Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
 1: Chambers Rd & E. 104th Ave

Year 2027 Background AM
 Chambers Rd Apartments Traffic Impact Study

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	 		 	 			 		 	 	
Traffic Volume (veh/h)	112	790	150	94	1040	207	275	196	125	242	165	221
Future Volume (veh/h)	112	790	150	94	1040	207	275	196	125	242	165	221
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	127	898	170	113	1253	0	312	223	142	266	181	243
Peak Hour Factor	0.88	0.88	0.88	0.83	0.83	0.83	0.88	0.88	0.88	0.91	0.91	0.91
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	186	1145	217	170	1349		336	555	339	721	419	374
Arrive On Green	0.05	0.38	0.38	0.05	0.38	0.00	0.10	0.26	0.26	0.08	0.24	0.24
Sat Flow, veh/h	3456	2981	564	3456	3554	1585	1781	2120	1295	3456	1777	1585
Grp Volume(v), veh/h	127	535	533	113	1253	0	312	185	180	266	181	243
Grp Sat Flow(s),veh/h/ln	1728	1777	1769	1728	1777	1585	1781	1777	1637	1728	1777	1585
Q Serve(g_s), s	4.3	31.8	31.9	3.9	40.5	0.0	12.3	10.3	10.9	6.9	10.4	16.6
Cycle Q Clear(g_c), s	4.3	31.8	31.9	3.9	40.5	0.0	12.3	10.3	10.9	6.9	10.4	16.6
Prop In Lane	1.00		0.32	1.00		1.00	1.00		0.79	1.00		1.00
Lane Grp Cap(c), veh/h	186	683	680	170	1349		336	465	429	721	419	374
V/C Ratio(X)	0.68	0.78	0.78	0.67	0.93		0.93	0.40	0.42	0.37	0.43	0.65
Avail Cap(c_a), veh/h	432	683	680	432	1349		336	465	429	812	419	374
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.86	0.86	0.86	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	55.8	32.6	32.6	56.1	35.7	0.0	38.1	36.5	36.7	31.0	39.0	41.4
Incr Delay (d2), s/veh	3.8	7.6	7.7	4.4	12.5	0.0	31.0	0.6	0.7	0.3	3.2	8.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.9	14.4	14.4	1.7	18.9	0.0	6.2	4.5	4.4	2.9	4.8	7.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	59.6	40.2	40.2	60.5	48.2	0.0	69.1	37.0	37.4	31.4	42.2	49.9
LnGrp LOS	E	D	D	E	D		E	D	D	C	D	D
Approach Vol, veh/h		1195			1366			677			690	
Approach Delay, s/veh		42.3			49.2			51.9			40.7	
Approach LOS		D			D			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.9	53.1	15.9	38.1	13.4	52.6	19.0	35.0				
Change Period (Y+Rc), s	7.0	7.0	6.7	6.7	7.0	7.0	6.7	6.7				
Max Green Setting (Gmax), s	15.0	37.0	12.3	28.3	15.0	37.0	12.3	28.3				
Max Q Clear Time (g_c+I1), s	5.9	33.9	8.9	12.9	6.3	42.5	14.3	18.6				
Green Ext Time (p_c), s	0.2	1.8	0.3	1.8	0.2	0.0	0.0	1.7				

Intersection Summary

HCM 6th Ctrl Delay	46.1
HCM 6th LOS	D

Notes

- User approved pedestrian interval to be less than phase max green.
- Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

Timings
2: Chambers Rd & 96th Ave



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	59	288	627	142	0	171	2
Future Volume (vph)	59	288	627	142	0	171	2
Turn Type	Prot	NA	NA	Perm	NA	Split	NA
Protected Phases	5	2	6		8	4	4
Permitted Phases				6			
Detector Phase	5	2	6	6	8	4	4
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Total Split (s)	23.0	59.0	36.0	36.0	23.0	23.0	23.0
Total Split (%)	21.9%	56.2%	34.3%	34.3%	21.9%	21.9%	21.9%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?	Yes		Yes	Yes			
Recall Mode	None	Min	None	None	None	None	None
Act Effect Green (s)	8.4	42.9	32.2	32.2	5.7	13.3	13.3
Actuated g/C Ratio	0.12	0.63	0.47	0.47	0.08	0.20	0.20
v/c Ratio	0.33	0.30	0.83	0.20	0.02	0.61	0.43
Control Delay	34.6	7.7	29.8	5.0	0.0	35.0	7.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	34.6	7.7	29.8	5.0	0.0	35.0	7.8
LOS	C	A	C	A	A	D	A
Approach Delay		12.3	25.2				21.6
Approach LOS		B	C				C

Intersection Summary

Cycle Length: 105
 Actuated Cycle Length: 68.1
 Natural Cycle: 115
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 21.1
 Intersection Capacity Utilization 73.5%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service D

Splits and Phases: 2: Chambers Rd & 96th Ave



Queues
2: Chambers Rd & 96th Ave



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	72	352	729	165	8	211	204
v/c Ratio	0.33	0.30	0.83	0.20	0.02	0.61	0.43
Control Delay	34.6	7.7	29.8	5.0	0.0	35.0	7.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	34.6	7.7	29.8	5.0	0.0	35.0	7.8
Queue Length 50th (ft)	28	53	256	5	0	80	1
Queue Length 95th (ft)	68	135	#618	43	0	154	40
Internal Link Dist (ft)		1966	2271		176		2115
Turn Bay Length (ft)	125			300			
Base Capacity (vph)	481	1496	881	824	722	481	578
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.24	0.83	0.20	0.01	0.44	0.35

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary

2: Chambers Rd & 96th Ave

Year 2027 Background AM
Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	59	288	1	0	627	142	0	0	3	171	2	164
Future Volume (veh/h)	59	288	1	0	627	142	0	0	3	171	2	164
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	72	351	1	0	729	165	0	0	8	211	2	202
Peak Hour Factor	0.82	0.82	0.82	0.86	0.86	0.86	0.38	0.38	0.38	0.81	0.81	0.81
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	137	1095	3	0	811	687	0	0	16	304	3	268
Arrive On Green	0.08	0.59	0.59	0.00	0.43	0.43	0.00	0.00	0.01	0.17	0.17	0.17
Sat Flow, veh/h	1781	1864	5	0	1870	1585	0	0	1585	1781	16	1572
Grp Volume(v), veh/h	72	0	352	0	729	165	0	0	8	211	0	204
Grp Sat Flow(s),veh/h/ln	1781	0	1869	0	1870	1585	0	0	1585	1781	0	1587
Q Serve(g_s), s	2.5	0.0	6.2	0.0	23.5	4.3	0.0	0.0	0.3	7.2	0.0	7.9
Cycle Q Clear(g_c), s	2.5	0.0	6.2	0.0	23.5	4.3	0.0	0.0	0.3	7.2	0.0	7.9
Prop In Lane	1.00		0.00	0.00		1.00	0.00		1.00	1.00		0.99
Lane Grp Cap(c), veh/h	137	0	1099	0	811	687	0	0	16	304	0	271
V/C Ratio(X)	0.52	0.00	0.32	0.00	0.90	0.24	0.00	0.00	0.49	0.69	0.00	0.75
Avail Cap(c_a), veh/h	494	0	1556	0	894	757	0	0	440	494	0	440
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	28.8	0.0	6.8	0.0	17.1	11.6	0.0	0.0	31.9	25.3	0.0	25.6
Incr Delay (d2), s/veh	3.1	0.0	0.2	0.0	11.3	0.2	0.0	0.0	20.7	2.8	0.0	4.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.1	0.0	2.0	0.0	11.3	1.4	0.0	0.0	0.2	3.0	0.0	3.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	31.9	0.0	7.0	0.0	28.3	11.8	0.0	0.0	52.6	28.1	0.0	29.8
LnGrp LOS	C	A	A	A	C	B	A	A	D	C	A	C
Approach Vol, veh/h		424			894			8				415
Approach Delay, s/veh		11.2			25.3			52.6				29.0
Approach LOS		B			C			D				C
Timer - Assigned Phs		2		4	5	6		8				
Phs Duration (G+Y+Rc), s		43.1		16.1	10.0	33.1		5.7				
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		5.0				
Max Green Setting (Gmax), s		54.0		18.0	18.0	31.0		18.0				
Max Q Clear Time (g_c+I1), s		8.2		9.9	4.5	25.5		2.3				
Green Ext Time (p_c), s		2.4		1.2	0.1	2.7		0.0				
Intersection Summary												
HCM 6th Ctrl Delay					22.8							
HCM 6th LOS					C							

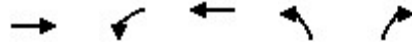
Intersection												
Int Delay, s/veh	9.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↶	↷		↶	↷		↶	↷		↶	↷	↶
Traffic Vol, veh/h	73	11	31	62	9	145	13	300	35	126	297	26
Future Vol, veh/h	73	11	31	62	9	145	13	300	35	126	297	26
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	150	-	-	100	-	-	235	-	-	235	-	135
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	79	12	34	67	10	158	14	326	38	137	323	28

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1054	989	323	1007	998	345	351	0	0	364	0	0
Stage 1	597	597	-	373	373	-	-	-	-	-	-	-
Stage 2	457	392	-	634	625	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	204	247	718	219	244	698	1208	-	-	1195	-	-
Stage 1	490	491	-	648	618	-	-	-	-	-	-	-
Stage 2	583	606	-	467	477	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	138	216	718	181	213	698	1208	-	-	1195	-	-
Mov Cap-2 Maneuver	138	216	-	181	213	-	-	-	-	-	-	-
Stage 1	484	435	-	640	611	-	-	-	-	-	-	-
Stage 2	439	599	-	383	422	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	44.2		19.7		0.3		2.4	
HCM LOS	E		C					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1208	-	-	138	446	181	616	1195	-	-
HCM Lane V/C Ratio	0.012	-	-	0.575	0.102	0.372	0.272	0.115	-	-
HCM Control Delay (s)	8	-	-	61.5	14	36.2	13	8.4	-	-
HCM Lane LOS	A	-	-	F	B	E	B	A	-	-
HCM 95th %tile Q(veh)	0	-	-	2.9	0.3	1.6	1.1	0.4	-	-

Timings
4: Sable Blvd & E. 104th Ave



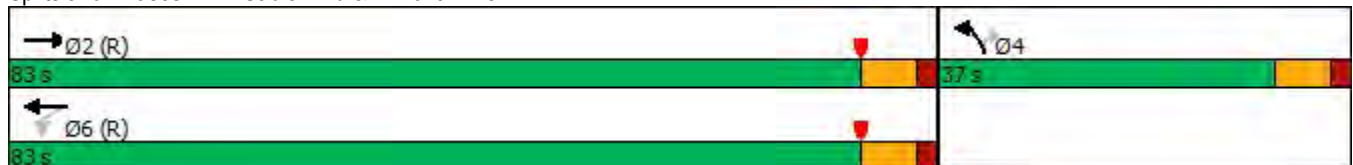
Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑	↑↑	↑	↑
Traffic Volume (vph)	1172	46	1428	180	81
Future Volume (vph)	1172	46	1428	180	81
Turn Type	NA	Perm	NA	Prot	Perm
Protected Phases	2		6	4	
Permitted Phases		6			4
Detector Phase	2	6	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	37.0	22.5	22.5	35.0	35.0
Total Split (s)	83.0	83.0	83.0	37.0	37.0
Total Split (%)	69.2%	69.2%	69.2%	30.8%	30.8%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	C-Max	C-Max	C-Max	None	None
Act Effct Green (s)	87.4	87.4	87.4	18.6	18.6
Actuated g/C Ratio	0.73	0.73	0.73	0.16	0.16
v/c Ratio	0.51	0.21	0.60	0.72	0.29
Control Delay	8.4	4.8	4.5	62.3	14.5
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	8.4	4.8	4.5	62.3	14.5
LOS	A	A	A	E	B
Approach Delay	8.4		4.5	47.5	
Approach LOS	A		A	D	

Intersection Summary

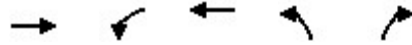
Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Yellow
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.72
 Intersection Signal Delay: 9.9
 Intersection Capacity Utilization 61.1%
 Analysis Period (min) 15

Intersection LOS: A
 ICU Level of Service B

Splits and Phases: 4: Sable Blvd & E. 104th Ave



Queues
4: Sable Blvd & E. 104th Ave



Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1308	50	1552	196	88
v/c Ratio	0.51	0.21	0.60	0.72	0.29
Control Delay	8.4	4.8	4.5	62.3	14.5
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	8.4	4.8	4.5	62.3	14.5
Queue Length 50th (ft)	201	6	104	146	9
Queue Length 95th (ft)	304	m9	m126	214	53
Internal Link Dist (ft)	491		3830	1801	
Turn Bay Length (ft)		300		70	
Base Capacity (vph)	2569	242	2578	442	452
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.51	0.21	0.60	0.44	0.19

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
4: Sable Blvd & E. 104th Ave

Year 2027 Background AM
Chambers Rd Apartments Traffic Impact Study



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	↵
Traffic Volume (veh/h)	1172	31	46	1428	180	81
Future Volume (veh/h)	1172	31	46	1428	180	81
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	1274	34	50	1552	196	88
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	2663	71	318	2676	232	207
Arrive On Green	0.75	0.75	0.75	0.75	0.13	0.13
Sat Flow, veh/h	3629	94	421	3647	1781	1585
Grp Volume(v), veh/h	640	668	50	1552	196	88
Grp Sat Flow(s),veh/h/ln	1777	1853	421	1777	1781	1585
Q Serve(g_s), s	16.7	16.7	6.3	23.0	12.9	6.1
Cycle Q Clear(g_c), s	16.7	16.7	23.0	23.0	12.9	6.1
Prop In Lane		0.05	1.00		1.00	1.00
Lane Grp Cap(c), veh/h	1338	1396	318	2676	232	207
V/C Ratio(X)	0.48	0.48	0.16	0.58	0.84	0.43
Avail Cap(c_a), veh/h	1338	1396	318	2676	445	396
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	0.13	0.13	1.00	1.00
Uniform Delay (d), s/veh	5.7	5.7	10.2	6.5	51.0	48.0
Incr Delay (d2), s/veh	1.2	1.2	0.1	0.1	8.1	1.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.8	6.0	0.6	7.4	6.3	2.5
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	6.9	6.9	10.3	6.6	59.1	49.4
LnGrp LOS	A	A	B	A	E	D
Approach Vol, veh/h	1308			1602	284	
Approach Delay, s/veh	6.9			6.7	56.1	
Approach LOS	A			A	E	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		97.4		22.6		97.4
Change Period (Y+Rc), s		7.0		7.0		7.0
Max Green Setting (Gmax), s		76.0		30.0		76.0
Max Q Clear Time (g_c+I1), s		18.7		14.9		25.0
Green Ext Time (p_c), s		13.1		0.7		21.0
Intersection Summary						
HCM 6th Ctrl Delay			11.2			
HCM 6th LOS			B			

Intersection												
Int Delay, s/veh	5.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Vol, veh/h	0	0	0	0	0	53	0	26	0	19	10	0
Future Vol, veh/h	0	0	0	0	0	53	0	26	0	19	10	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	0	58	0	28	0	21	11	0

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	110	81	11	81	81	28	11	0	0	28	0	0
Stage 1	53	53	-	28	28	-	-	-	-	-	-	-
Stage 2	57	28	-	53	53	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	868	809	1070	907	809	1047	1608	-	-	1585	-	-
Stage 1	960	851	-	989	872	-	-	-	-	-	-	-
Stage 2	955	872	-	960	851	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	812	798	1070	898	798	1047	1608	-	-	1585	-	-
Mov Cap-2 Maneuver	812	798	-	898	798	-	-	-	-	-	-	-
Stage 1	960	840	-	989	872	-	-	-	-	-	-	-
Stage 2	902	872	-	948	840	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	0		8.6		0		4.8	
HCM LOS	A		A					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1608	-	-	-	1047	1585	-	-
HCM Lane V/C Ratio	-	-	-	-	0.055	0.013	-	-
HCM Control Delay (s)	0	-	-	0	8.6	7.3	0	-
HCM Lane LOS	A	-	-	A	A	A	A	-
HCM 95th %tile Q(veh)	0	-	-	-	0.2	0	-	-

Timings 1: Chambers Rd & E. 104th Ave

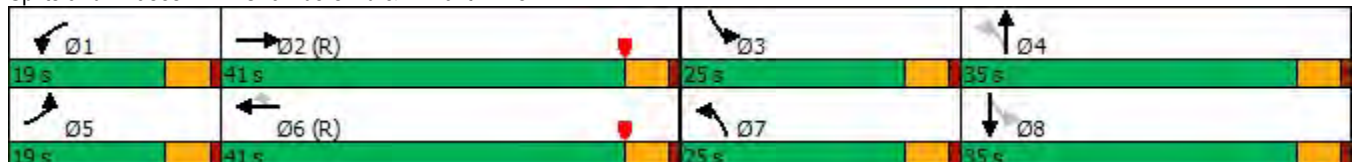
Year 2027 Background PM
Chambers Rd Apartments Traffic Impact Study

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	239	874	116	879	277	262	300	335	306
Future Volume (vph)	239	874	116	879	277	262	300	335	306
Turn Type	Prot	NA	Prot	NA	Perm	pm+pt	NA	pm+pt	NA
Protected Phases	5	2	1	6		7	4	3	8
Permitted Phases					6	4		8	
Detector Phase	5	2	1	6	6	7	4	3	8
Switch Phase									
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	12.5	24.0	11.0	24.5	24.5	11.0	24.0	11.0	24.5
Total Split (s)	19.0	41.0	19.0	41.0	41.0	25.0	35.0	25.0	35.0
Total Split (%)	15.8%	34.2%	15.8%	34.2%	34.2%	20.8%	29.2%	20.8%	29.2%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Min	None	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	14.6	50.1	10.6	46.1	46.1	42.9	24.2	35.8	20.6
Actuated g/C Ratio	0.12	0.42	0.09	0.38	0.38	0.36	0.20	0.30	0.17
v/c Ratio	0.66	0.86	0.50	0.84	0.43	0.83	0.66	0.60	0.75
Control Delay	61.8	34.8	57.5	41.8	4.9	49.1	43.2	30.8	51.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.8	34.8	57.5	41.8	4.9	49.1	43.2	30.8	51.3
LOS	E	C	E	D	A	D	D	C	D
Approach Delay		39.7		35.2			45.4		42.1
Approach LOS		D		D			D		D

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 1 (1%), Referenced to phase 2:EBT and 6:WBT, Start of Yellow
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.86
 Intersection Signal Delay: 39.5
 Intersection LOS: D
 Intersection Capacity Utilization 78.1%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 1: Chambers Rd & E. 104th Ave



Queues
1: Chambers Rd & E. 104th Ave































Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	275	1250	151	1142	360	291	474	381	465
v/c Ratio	0.66	0.86	0.50	0.84	0.43	0.83	0.66	0.60	0.75
Control Delay	61.8	34.8	57.5	41.8	4.9	49.1	43.2	30.8	51.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.8	34.8	57.5	41.8	4.9	49.1	43.2	30.8	51.3
Queue Length 50th (ft)	115	331	58	427	0	166	157	105	168
Queue Length 95th (ft)	155	#430	77	#493	27	#263	207	127	208
Internal Link Dist (ft)		3830		955			548		948
Turn Bay Length (ft)	275		380		350	290		285	
Base Capacity (vph)	438	1448	400	1359	829	368	884	772	879
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.63	0.86	0.38	0.84	0.43	0.79	0.54	0.49	0.53

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
 1: Chambers Rd & E. 104th Ave

Year 2027 Background PM
 Chambers Rd Apartments Traffic Impact Study

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	 		 	 			 		 	 	
Traffic Volume (veh/h)	239	874	213	116	879	277	262	300	127	335	306	103
Future Volume (veh/h)	239	874	213	116	879	277	262	300	127	335	306	103
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	275	1005	245	151	1142	0	291	333	141	381	348	117
Peak Hour Factor	0.87	0.87	0.87	0.77	0.77	0.77	0.90	0.90	0.90	0.88	0.88	0.88
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	334	1299	316	211	1502		359	492	204	640	430	142
Arrive On Green	0.10	0.46	0.46	0.06	0.42	0.00	0.15	0.20	0.20	0.11	0.16	0.16
Sat Flow, veh/h	3456	2835	689	3456	3554	1585	1781	2447	1017	3456	2623	868
Grp Volume(v), veh/h	275	629	621	151	1142	0	291	240	234	381	234	231
Grp Sat Flow(s),veh/h/ln	1728	1777	1746	1728	1777	1585	1781	1777	1687	1728	1777	1714
Q Serve(g_s), s	9.4	35.6	35.9	5.1	32.8	0.0	15.8	15.0	15.4	10.7	15.2	15.6
Cycle Q Clear(g_c), s	9.4	35.6	35.9	5.1	32.8	0.0	15.8	15.0	15.4	10.7	15.2	15.6
Prop In Lane	1.00		0.39	1.00		1.00	1.00		0.60	1.00		0.51
Lane Grp Cap(c), veh/h	334	814	800	211	1502		359	357	339	640	292	281
V/C Ratio(X)	0.82	0.77	0.78	0.72	0.76		0.81	0.67	0.69	0.60	0.80	0.82
Avail Cap(c_a), veh/h	403	814	800	403	1502		388	444	422	825	444	429
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.73	0.73	0.73	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.2	27.3	27.3	55.3	29.5	0.0	34.2	44.3	44.5	35.9	48.3	48.5
Incr Delay (d2), s/veh	8.3	5.2	5.4	4.5	3.7	0.0	11.6	2.8	3.5	0.9	6.1	7.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.3	15.3	15.2	2.3	14.0	0.0	7.8	6.7	6.7	4.5	7.1	7.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	61.5	32.5	32.7	59.8	33.2	0.0	45.8	47.1	47.9	36.8	54.3	55.9
LnGrp LOS	E	C	C	E	C		D	D	D	D	D	E
Approach Vol, veh/h		1525			1293			765			846	
Approach Delay, s/veh		37.8			36.3			46.9			46.9	
Approach LOS		D			D			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.3	60.0	18.6	29.1	16.6	55.7	23.0	24.7				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	14.0	36.0	20.0	30.0	14.0	36.0	20.0	30.0				
Max Q Clear Time (g_c+I1), s	7.1	37.9	12.7	17.4	11.4	34.8	17.8	17.6				
Green Ext Time (p_c), s	0.2	0.0	0.8	2.1	0.2	0.8	0.2	2.1				

Intersection Summary

HCM 6th Ctrl Delay	40.6
HCM 6th LOS	D

Notes

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

Timings
2: Chambers Rd & 96th Ave



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	237	749	569	159	1	173	1
Future Volume (vph)	237	749	569	159	1	173	1
Turn Type	Prot	NA	NA	Perm	NA	Split	NA
Protected Phases	5	2	6		8	4	4
Permitted Phases				6			
Detector Phase	5	2	6	6	8	4	4
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Total Split (s)	23.0	49.0	26.0	26.0	23.0	23.0	23.0
Total Split (%)	24.2%	51.6%	27.4%	27.4%	24.2%	24.2%	24.2%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?	Yes		Yes	Yes			
Recall Mode	None	Min	None	None	None	None	None
Act Effect Green (s)	14.2	40.8	21.5	21.5	5.8	12.6	12.6
Actuated g/C Ratio	0.22	0.62	0.33	0.33	0.09	0.19	0.19
v/c Ratio	0.64	0.66	0.98	0.26	0.03	0.59	0.21
Control Delay	32.8	13.3	58.8	5.5	28.0	32.9	8.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	32.8	13.3	58.8	5.5	28.0	32.9	8.6
LOS	C	B	E	A	C	C	A
Approach Delay		18.0	47.1		28.0		26.0
Approach LOS		B	D		C		C

Intersection Summary

Cycle Length: 95
 Actuated Cycle Length: 65.3
 Natural Cycle: 105
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 0.98
 Intersection Signal Delay: 29.9
 Intersection Capacity Utilization 98.1%
 Analysis Period (min) 15

Intersection LOS: C
 ICU Level of Service F

Splits and Phases: 2: Chambers Rd & 96th Ave



Queues
2: Chambers Rd & 96th Ave



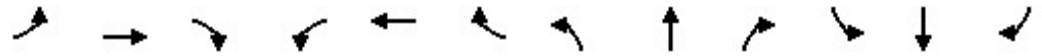
Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	244	772	599	167	4	199	78
v/c Ratio	0.64	0.66	0.98	0.26	0.03	0.59	0.21
Control Delay	32.8	13.3	58.8	5.5	28.0	32.9	8.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	32.8	13.3	58.8	5.5	28.0	32.9	8.6
Queue Length 50th (ft)	84	152	222	0	1	69	0
Queue Length 95th (ft)	194	463	#599	46	6	153	32
Internal Link Dist (ft)		1966	2271		176		2115
Turn Bay Length (ft)	125			300			
Base Capacity (vph)	498	1283	612	632	490	498	502
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.49	0.60	0.98	0.26	0.01	0.40	0.16

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
 2: Chambers Rd & 96th Ave

Year 2027 Background PM
 Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	237	749	0	0	569	159	0	1	1	173	1	67
Future Volume (veh/h)	237	749	0	0	569	159	0	1	1	173	1	67
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	244	772	0	0	599	167	0	2	2	199	1	77
Peak Hour Factor	0.97	0.97	0.97	0.95	0.95	0.95	0.50	0.50	0.50	0.87	0.87	0.87
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	306	1118	0	0	643	545	0	5	5	266	3	234
Arrive On Green	0.17	0.60	0.00	0.00	0.34	0.34	0.00	0.01	0.01	0.15	0.15	0.15
Sat Flow, veh/h	1781	1870	0	0	1870	1585	0	858	858	1781	20	1568
Grp Volume(v), veh/h	244	772	0	0	599	167	0	0	4	199	0	78
Grp Sat Flow(s),veh/h/ln	1781	1870	0	0	1870	1585	0	0	1716	1781	0	1588
Q Serve(g_s), s	8.0	17.1	0.0	0.0	18.8	4.7	0.0	0.0	0.1	6.5	0.0	2.7
Cycle Q Clear(g_c), s	8.0	17.1	0.0	0.0	18.8	4.7	0.0	0.0	0.1	6.5	0.0	2.7
Prop In Lane	1.00		0.00	0.00		1.00	0.00		0.50	1.00		0.99
Lane Grp Cap(c), veh/h	306	1118	0	0	643	545	0	0	9	266	0	237
V/C Ratio(X)	0.80	0.69	0.00	0.00	0.93	0.31	0.00	0.00	0.43	0.75	0.00	0.33
Avail Cap(c_a), veh/h	528	1356	0	0	647	548	0	0	509	528	0	471
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.1	8.4	0.0	0.0	19.2	14.6	0.0	0.0	30.1	24.7	0.0	23.1
Incr Delay (d2), s/veh	4.8	1.2	0.0	0.0	20.2	0.3	0.0	0.0	29.0	4.2	0.0	0.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.5	5.5	0.0	0.0	10.8	1.6	0.0	0.0	0.1	2.8	0.0	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	28.9	9.5	0.0	0.0	39.4	14.9	0.0	0.0	59.1	28.9	0.0	23.9
LnGrp LOS	C	A	A	A	D	B	A	A	E	C	A	C
Approach Vol, veh/h		1016			766			4				277
Approach Delay, s/veh		14.2			34.1			59.1				27.5
Approach LOS		B			C			E				C
Timer - Assigned Phs		2		4	5	6		8				
Phs Duration (G+Y+Rc), s		41.3		14.1	15.4	25.9		5.3				
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		5.0				
Max Green Setting (Gmax), s		44.0		18.0	18.0	21.0		18.0				
Max Q Clear Time (g_c+l1), s		19.1		8.5	10.0	20.8		2.1				
Green Ext Time (p_c), s		6.1		0.7	0.4	0.1		0.0				
Intersection Summary												
HCM 6th Ctrl Delay				23.4								
HCM 6th LOS				C								

Intersection												
Int Delay, s/veh	10.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↶	↷		↶	↷		↶	↷		↶	↷	↶
Traffic Vol, veh/h	51	5	17	33	8	132	46	403	71	172	327	80
Future Vol, veh/h	51	5	17	33	8	132	46	403	71	172	327	80
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	150	-	-	100	-	-	235	-	-	235	-	135
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	55	5	18	36	9	143	50	438	77	187	355	87

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1382	1344	355	1361	1393	477	442	0	0	515	0	0
Stage 1	729	729	-	577	577	-	-	-	-	-	-	-
Stage 2	653	615	-	784	816	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	121	152	689	125	142	588	1118	-	-	1051	-	-
Stage 1	414	428	-	502	502	-	-	-	-	-	-	-
Stage 2	456	482	-	386	391	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	72	119	689	98	111	588	1118	-	-	1051	-	-
Mov Cap-2 Maneuver	72	119	-	98	111	-	-	-	-	-	-	-
Stage 1	395	352	-	479	479	-	-	-	-	-	-	-
Stage 2	323	460	-	304	321	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	106.2		24.9		0.7		2.7	
HCM LOS	F		C					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1118	-	-	72	330	98	472	1051	-	-
HCM Lane V/C Ratio	0.045	-	-	0.77	0.072	0.366	0.322	0.178	-	-
HCM Control Delay (s)	8.4	-	-	144.7	16.8	61.6	16.2	9.2	-	-
HCM Lane LOS	A	-	-	F	C	F	C	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	3.6	0.2	1.5	1.4	0.6	-	-

Timings
4: Sable Blvd & E. 104th Ave

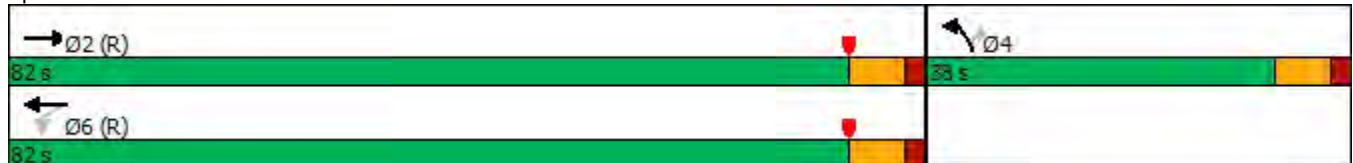
	→	↙	←	↘	↗
Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↙	↑↑	↙	↗
Traffic Volume (vph)	1540	99	1323	92	61
Future Volume (vph)	1540	99	1323	92	61
Turn Type	NA	Perm	NA	Prot	Perm
Protected Phases	2		6	4	
Permitted Phases		6			4
Detector Phase	2	6	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	37.0	25.0	25.0	35.0	35.0
Total Split (s)	82.0	82.0	82.0	38.0	38.0
Total Split (%)	68.3%	68.3%	68.3%	31.7%	31.7%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	C-Max	C-Max	C-Max	None	None
Act Effct Green (s)	93.9	93.9	93.9	12.1	12.1
Actuated g/C Ratio	0.78	0.78	0.78	0.10	0.10
v/c Ratio	0.65	0.73	0.52	0.56	0.35
Control Delay	7.5	33.6	3.0	62.7	35.2
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	7.5	33.6	3.0	62.7	35.2
LOS	A	C	A	E	D
Approach Delay	7.5		5.1	51.8	
Approach LOS	A		A	D	

Intersection Summary

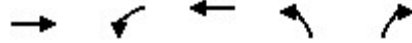
Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Yellow
 Natural Cycle: 140
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.73
 Intersection Signal Delay: 8.5
 Intersection Capacity Utilization 73.7%
 Analysis Period (min) 15

Intersection LOS: A
 ICU Level of Service D

Splits and Phases: 4: Sable Blvd & E. 104th Ave



Queues
4: Sable Blvd & E. 104th Ave



Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1779	108	1438	100	66
v/c Ratio	0.65	0.73	0.52	0.56	0.35
Control Delay	7.5	33.6	3.0	62.7	35.2
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	7.5	33.6	3.0	62.7	35.2
Queue Length 50th (ft)	263	20	64	75	27
Queue Length 95th (ft)	394	m58	78	128	69
Internal Link Dist (ft)	491		3830	1801	
Turn Bay Length (ft)		300		70	
Base Capacity (vph)	2745	147	2768	457	430
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.65	0.73	0.52	0.22	0.15

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
4: Sable Blvd & E. 104th Ave

Year 2027 Background PM
Chambers Rd Apartments Traffic Impact Study



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	↵
Traffic Volume (veh/h)	1540	97	99	1323	92	61
Future Volume (veh/h)	1540	97	99	1323	92	61
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	1674	105	108	1438	100	66
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	2746	171	225	2872	134	119
Arrive On Green	0.81	0.81	0.81	0.81	0.08	0.08
Sat Flow, veh/h	3491	212	267	3647	1781	1585
Grp Volume(v), veh/h	870	909	108	1438	100	66
Grp Sat Flow(s),veh/h/ln	1777	1832	267	1777	1781	1585
Q Serve(g_s), s	22.1	22.7	31.0	15.6	6.6	4.8
Cycle Q Clear(g_c), s	22.1	22.7	53.7	15.6	6.6	4.8
Prop In Lane		0.12	1.00		1.00	1.00
Lane Grp Cap(c), veh/h	1436	1481	225	2872	134	119
V/C Ratio(X)	0.61	0.61	0.48	0.50	0.75	0.55
Avail Cap(c_a), veh/h	1436	1481	225	2872	460	409
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.43	0.43	1.00	1.00
Uniform Delay (d), s/veh	4.3	4.4	14.6	3.7	54.4	53.6
Incr Delay (d2), s/veh	1.9	1.9	3.1	0.3	8.0	4.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.7	7.2	2.0	4.4	3.3	2.1
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	6.2	6.3	17.7	4.0	62.4	57.5
LnGrp LOS	A	A	B	A	E	E
Approach Vol, veh/h	1779			1546	166	
Approach Delay, s/veh	6.3			4.9	60.5	
Approach LOS	A			A	E	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		104.0		16.0		104.0
Change Period (Y+Rc), s		7.0		7.0		7.0
Max Green Setting (Gmax), s		75.0		31.0		75.0
Max Q Clear Time (g_c+I1), s		24.7		8.6		55.7
Green Ext Time (p_c), s		23.3		0.4		13.1
Intersection Summary						
HCM 6th Ctrl Delay			8.3			
HCM 6th LOS			A			

Intersection												
Int Delay, s/veh	5.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	0	0	0	0	38	0	19	0	61	31	0
Future Vol, veh/h	0	0	0	0	0	38	0	19	0	61	31	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	0	41	0	21	0	66	34	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	208	187	34	187	187	21	34	0	0	21	0	0
Stage 1	166	166	-	21	21	-	-	-	-	-	-	-
Stage 2	42	21	-	166	166	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	749	708	1039	774	708	1056	1578	-	-	1595	-	-
Stage 1	836	761	-	998	878	-	-	-	-	-	-	-
Stage 2	972	878	-	836	761	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	697	678	1039	749	678	1056	1578	-	-	1595	-	-
Mov Cap-2 Maneuver	697	678	-	749	678	-	-	-	-	-	-	-
Stage 1	836	729	-	998	878	-	-	-	-	-	-	-
Stage 2	934	878	-	801	729	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	8.5	0	4.9
HCM LOS	A	A		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1578	-	-	-	1056	1595	-	-
HCM Lane V/C Ratio	-	-	-	-	0.039	0.042	-	-
HCM Control Delay (s)	0	-	-	0	8.5	7.4	0	-
HCM Lane LOS	A	-	-	A	A	A	A	-
HCM 95th %tile Q(veh)	0	-	-	-	0.1	0.1	-	-

Timings
1: Chambers Rd & E. 104th Ave

Year 2027 w/Project AM
Chambers Rd Apartments Traffic Impact Study

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	112	790	100	1040	207	305	200	242	166
Future Volume (vph)	112	790	100	1040	207	305	200	242	166
Turn Type	Prot	NA	Prot	NA	Perm	pm+pt	NA	pm+pt	NA
Protected Phases	5	2	1	6		7	4	3	8
Permitted Phases					6	4		8	
Detector Phase	5	2	1	6	6	7	4	3	8
Switch Phase									
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	12.0	37.0	12.0	37.0	37.0	12.0	34.7	11.7	34.7
Total Split (s)	12.0	52.3	13.0	53.3	53.3	20.0	41.0	13.7	34.7
Total Split (%)	10.0%	43.6%	10.8%	44.4%	44.4%	16.7%	34.2%	11.4%	28.9%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	4.7	4.7	4.7	4.7
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0	6.7	6.7	6.7	6.7
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Min	None	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	9.9	50.9	9.4	50.4	50.4	38.7	24.7	24.8	17.2
Actuated g/C Ratio	0.08	0.42	0.08	0.42	0.42	0.32	0.21	0.21	0.14
v/c Ratio	0.45	0.73	0.45	0.84	0.31	1.18	0.49	0.55	0.76
Control Delay	76.6	24.8	57.9	38.2	4.0	143.7	27.0	36.1	44.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	76.6	24.8	57.9	38.2	4.0	143.7	27.0	36.1	44.5
LOS	E	C	E	D	A	F	C	D	D
Approach Delay		30.2		34.4			81.9		41.3
Approach LOS		C		C			F		D

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 8 (7%), Referenced to phase 2:EBT and 6:WBT, Start of Yellow
 Natural Cycle: 100
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.18
 Intersection Signal Delay: 42.6
 Intersection LOS: D
 Intersection Capacity Utilization 84.3%
 ICU Level of Service E
 Analysis Period (min) 15

Splits and Phases: 1: Chambers Rd & E. 104th Ave



Queues
1: Chambers Rd & E. 104th Ave

Year 2027 w/Project AM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	127	1079	120	1253	249	347	390	266	425
v/c Ratio	0.45	0.73	0.45	0.84	0.31	1.18	0.49	0.55	0.76
Control Delay	76.6	24.8	57.9	38.2	4.0	143.7	27.0	36.1	44.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	76.6	24.8	57.9	38.2	4.0	143.7	27.0	36.1	44.5
Queue Length 50th (ft)	54	347	46	442	0	~285	85	79	124
Queue Length 95th (ft)	86	462	71	511	38	#429	122	105	171
Internal Link Dist (ft)		2590		955			548		948
Turn Bay Length (ft)	275		380		350	290		285	
Base Capacity (vph)	284	1477	268	1485	808	294	1055	486	842
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.45	0.73	0.45	0.84	0.31	1.18	0.37	0.55	0.50

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.





























Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
 1: Chambers Rd & E. 104th Ave

Year 2027 w/Project AM
 Chambers Rd Apartments Traffic Impact Study

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	 		 	 			 		 	 	
Traffic Volume (veh/h)	112	790	159	100	1040	207	305	200	143	242	166	221
Future Volume (veh/h)	112	790	159	100	1040	207	305	200	143	242	166	221
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	127	898	181	120	1253	0	347	227	162	266	182	243
Peak Hour Factor	0.88	0.88	0.88	0.83	0.83	0.83	0.88	0.88	0.88	0.91	0.91	0.91
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	144	1275	257	172	1567		286	466	318	569	317	282
Arrive On Green	0.08	0.87	0.87	0.05	0.44	0.00	0.11	0.23	0.23	0.06	0.18	0.18
Sat Flow, veh/h	3456	2947	594	3456	3554	1585	1781	2019	1380	3456	1777	1585
Grp Volume(v), veh/h	127	541	538	120	1253	0	347	199	190	266	182	243
Grp Sat Flow(s),veh/h/ln	1728	1777	1763	1728	1777	1585	1781	1777	1622	1728	1777	1585
Q Serve(g_s), s	4.4	12.6	12.6	4.1	36.5	0.0	13.3	11.6	12.3	7.0	11.3	17.9
Cycle Q Clear(g_c), s	4.4	12.6	12.6	4.1	36.5	0.0	13.3	11.6	12.3	7.0	11.3	17.9
Prop In Lane	1.00		0.34	1.00		1.00	1.00		0.85	1.00		1.00
Lane Grp Cap(c), veh/h	144	769	763	172	1567		286	410	374	569	317	282
V/C Ratio(X)	0.88	0.70	0.70	0.70	0.80		1.21	0.48	0.51	0.47	0.57	0.86
Avail Cap(c_a), veh/h	144	769	763	173	1567		286	508	464	569	415	370
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.86	0.86	0.86	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	54.7	5.4	5.4	56.1	29.0	0.0	39.3	40.0	40.2	38.5	45.1	47.9
Incr Delay (d2), s/veh	38.1	4.6	4.7	11.5	4.4	0.0	124.3	0.9	1.1	0.6	1.6	14.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.5	3.1	3.1	2.0	15.5	0.0	11.9	5.1	4.9	3.2	5.0	8.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	92.8	10.1	10.1	67.6	33.3	0.0	163.7	40.9	41.3	39.1	46.8	62.6
LnGrp LOS	F	B	B	E	C		F	D	D	D	D	E
Approach Vol, veh/h		1206			1373			736			691	
Approach Delay, s/veh		18.8			36.3			98.9			49.4	
Approach LOS		B			D			F			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	13.0	58.9	13.7	34.4	12.0	59.9	20.0	28.1				
Change Period (Y+Rc), s	7.0	7.0	6.7	6.7	7.0	7.0	6.7	6.7				
Max Green Setting (Gmax), s	6.0	45.3	7.0	34.3	5.0	46.3	13.3	28.0				
Max Q Clear Time (g_c+I1), s	6.1	14.6	9.0	14.3	6.4	38.5	15.3	19.9				
Green Ext Time (p_c), s	0.0	7.5	0.0	2.1	0.0	4.6	0.0	1.5				

Intersection Summary

HCM 6th Ctrl Delay	44.8
HCM 6th LOS	D

Notes

- User approved pedestrian interval to be less than phase max green.
- Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

Timings
2: Chambers Rd & 96th Ave

Year 2027 w/Project AM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	88	288	627	155	0	205	2
Future Volume (vph)	88	288	627	155	0	205	2
Turn Type	Prot	NA	NA	Perm	NA	Split	NA
Protected Phases	5	2	6		8	4	4
Permitted Phases				6			
Detector Phase	5	2	6	6	8	4	4
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Total Split (s)	23.0	59.0	36.0	36.0	23.0	23.0	23.0
Total Split (%)	21.9%	56.2%	34.3%	34.3%	21.9%	21.9%	21.9%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?	Yes		Yes	Yes			
Recall Mode	None	Min	None	None	None	None	None
Act Effct Green (s)	9.9	43.8	31.9	31.9	5.7	16.0	16.0
Actuated g/C Ratio	0.14	0.61	0.44	0.44	0.08	0.22	0.22
v/c Ratio	0.44	0.31	0.88	0.23	0.02	0.64	0.52
Control Delay	37.1	8.3	36.5	5.4	0.0	36.1	7.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	37.1	8.3	36.5	5.4	0.0	36.1	7.4
LOS	D	A	D	A	A	D	A
Approach Delay		15.0	30.3				20.4
Approach LOS		B	C				C

Intersection Summary

Cycle Length: 105
 Actuated Cycle Length: 71.8
 Natural Cycle: 115
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 0.88
 Intersection Signal Delay: 23.7
 Intersection LOS: C
 Intersection Capacity Utilization 78.7%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 2: Chambers Rd & 96th Ave



Queues
2: Chambers Rd & 96th Ave

Year 2027 w/Project AM
Chambers Rd Apartments Traffic Impact Study



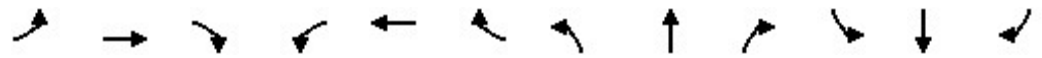
Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	107	352	729	180	8	253	307
v/c Ratio	0.44	0.31	0.88	0.23	0.02	0.64	0.52
Control Delay	37.1	8.3	36.5	5.4	0.0	36.1	7.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	37.1	8.3	36.5	5.4	0.0	36.1	7.4
Queue Length 50th (ft)	46	66	306	6	0	102	1
Queue Length 95th (ft)	93	134	#643	46	0	190	44
Internal Link Dist (ft)		1966	2271		176		2572
Turn Bay Length (ft)	70			300			
Base Capacity (vph)	457	1432	828	790	692	457	635
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.23	0.25	0.88	0.23	0.01	0.55	0.48

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
 2: Chambers Rd & 96th Ave

Year 2027 w/Project AM
 Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	88	288	1	0	627	155	0	0	3	205	2	247
Future Volume (veh/h)	88	288	1	0	627	155	0	0	3	205	2	247
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	107	351	1	0	729	180	0	0	8	253	2	305
Peak Hour Factor	0.82	0.82	0.82	0.86	0.86	0.86	0.38	0.38	0.38	0.81	0.81	0.81
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	156	1055	3	0	769	652	0	0	16	395	2	350
Arrive On Green	0.09	0.57	0.57	0.00	0.41	0.41	0.00	0.00	0.01	0.22	0.22	0.22
Sat Flow, veh/h	1781	1864	5	0	1870	1585	0	0	1585	1781	10	1576
Grp Volume(v), veh/h	107	0	352	0	729	180	0	0	8	253	0	307
Grp Sat Flow(s),veh/h/ln	1781	0	1869	0	1870	1585	0	0	1585	1781	0	1587
Q Serve(g_s), s	4.3	0.0	7.5	0.0	27.9	5.6	0.0	0.0	0.4	9.6	0.0	13.9
Cycle Q Clear(g_c), s	4.3	0.0	7.5	0.0	27.9	5.6	0.0	0.0	0.4	9.6	0.0	13.9
Prop In Lane	1.00		0.00	0.00		1.00	0.00		1.00	1.00		0.99
Lane Grp Cap(c), veh/h	156	0	1058	0	769	652	0	0	16	395	0	352
V/C Ratio(X)	0.69	0.00	0.33	0.00	0.95	0.28	0.00	0.00	0.49	0.64	0.00	0.87
Avail Cap(c_a), veh/h	431	0	1358	0	780	661	0	0	384	431	0	384
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	32.9	0.0	8.6	0.0	21.1	14.5	0.0	0.0	36.6	26.2	0.0	27.9
Incr Delay (d2), s/veh	5.2	0.0	0.2	0.0	20.4	0.2	0.0	0.0	21.3	2.8	0.0	18.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.0	0.0	2.7	0.0	15.4	1.9	0.0	0.0	0.2	4.1	0.0	6.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	38.2	0.0	8.8	0.0	41.5	14.8	0.0	0.0	57.9	29.0	0.0	46.1
LnGrp LOS	D	A	A	A	D	B	A	A	E	C	A	D
Approach Vol, veh/h		459			909			8				560
Approach Delay, s/veh		15.6			36.2			57.9				38.4
Approach LOS		B			D			E				D
Timer - Assigned Phs		2		4	5	6		8				
Phs Duration (G+Y+Rc), s		47.1		21.5	11.5	35.6		5.8				
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		5.0				
Max Green Setting (Gmax), s		54.0		18.0	18.0	31.0		18.0				
Max Q Clear Time (g_c+I1), s		9.5		15.9	6.3	29.9		2.4				
Green Ext Time (p_c), s		2.4		0.6	0.2	0.6		0.0				
Intersection Summary												
HCM 6th Ctrl Delay				32.1								
HCM 6th LOS				C								

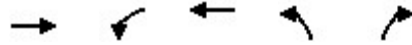
Intersection												
Int Delay, s/veh	9.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔		↔	↔		↔	↔		↔	↔	↔
Traffic Vol, veh/h	73	11	31	62	9	145	13	308	35	126	323	26
Future Vol, veh/h	73	11	31	62	9	145	13	308	35	126	323	26
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	150	-	-	100	-	-	235	-	-	235	-	135
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	79	12	34	67	10	158	14	335	38	137	351	28

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1091	1026	351	1044	1035	354	379	0	0	373	0	0
Stage 1	625	625	-	382	382	-	-	-	-	-	-	-
Stage 2	466	401	-	662	653	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	192	235	692	207	232	690	1179	-	-	1185	-	-
Stage 1	473	477	-	640	613	-	-	-	-	-	-	-
Stage 2	577	601	-	451	464	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	129	205	692	170	203	690	1179	-	-	1185	-	-
Mov Cap-2 Maneuver	129	205	-	170	203	-	-	-	-	-	-	-
Stage 1	467	422	-	632	606	-	-	-	-	-	-	-
Stage 2	433	594	-	369	410	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	49.5		20.7		0.3		2.2	
HCM LOS	E		C					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1179	-	-	129	427	170	605	1185	-	-
HCM Lane V/C Ratio	0.012	-	-	0.615	0.107	0.396	0.277	0.116	-	-
HCM Control Delay (s)	8.1	-	-	69.7	14.4	39.4	13.2	8.4	-	-
HCM Lane LOS	A	-	-	F	B	E	B	A	-	-
HCM 95th %tile Q(veh)	0	-	-	3.2	0.4	1.7	1.1	0.4	-	-

Timings
4: Sable Blvd & E. 104th Ave

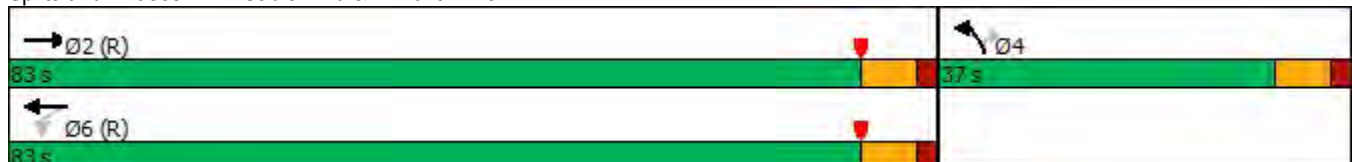


Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑	↑↑	↑	↑
Traffic Volume (vph)	1181	46	1458	188	81
Future Volume (vph)	1181	46	1458	188	81
Turn Type	NA	Perm	NA	Prot	Perm
Protected Phases	2		6	4	
Permitted Phases		6			4
Detector Phase	2	6	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	37.0	25.0	25.0	35.0	35.0
Total Split (s)	83.0	83.0	83.0	37.0	37.0
Total Split (%)	69.2%	69.2%	69.2%	30.8%	30.8%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	C-Max	C-Max	C-Max	None	None
Act Effct Green (s)	86.9	86.9	86.9	19.1	19.1
Actuated g/C Ratio	0.72	0.72	0.72	0.16	0.16
v/c Ratio	0.52	0.21	0.62	0.73	0.28
Control Delay	8.7	5.0	10.6	62.2	14.8
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	8.7	5.0	10.6	62.2	14.8
LOS	A	A	B	E	B
Approach Delay	8.7		10.4	47.9	
Approach LOS	A		B	D	

Intersection Summary

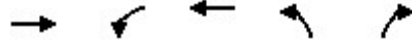
Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Yellow
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.73
 Intersection Signal Delay: 13.1
 Intersection Capacity Utilization 62.4%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service B

Splits and Phases: 4: Sable Blvd & E. 104th Ave



Queues
4: Sable Blvd & E. 104th Ave

Year 2027 w/Project AM
Chambers Rd Apartments Traffic Impact Study



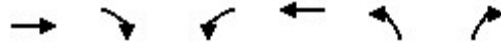
Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1320	50	1585	204	88
v/c Ratio	0.52	0.21	0.62	0.73	0.28
Control Delay	8.7	5.0	10.6	62.2	14.8
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	8.7	5.0	10.6	62.2	14.8
Queue Length 50th (ft)	208	10	462	152	10
Queue Length 95th (ft)	314	m14	m511	221	54
Internal Link Dist (ft)	443		2590	1245	
Turn Bay Length (ft)		300		70	
Base Capacity (vph)	2553	236	2562	442	450
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.52	0.21	0.62	0.46	0.20

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
4: Sable Blvd & E. 104th Ave

Year 2027 w/Project AM
Chambers Rd Apartments Traffic Impact Study



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↙	↑↑	↙	↗
Traffic Volume (veh/h)	1181	33	46	1458	188	81
Future Volume (veh/h)	1181	33	46	1458	188	81
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	1284	36	50	1585	204	88
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	2643	74	311	2660	240	214
Arrive On Green	0.75	0.75	0.50	0.50	0.13	0.13
Sat Flow, veh/h	3624	99	416	3647	1781	1585
Grp Volume(v), veh/h	646	674	50	1585	204	88
Grp Sat Flow(s),veh/h/ln	1777	1853	416	1777	1781	1585
Q Serve(g_s), s	17.2	17.3	9.3	38.1	13.4	6.1
Cycle Q Clear(g_c), s	17.2	17.3	26.6	38.1	13.4	6.1
Prop In Lane		0.05	1.00		1.00	1.00
Lane Grp Cap(c), veh/h	1330	1387	311	2660	240	214
V/C Ratio(X)	0.49	0.49	0.16	0.60	0.85	0.41
Avail Cap(c_a), veh/h	1330	1387	311	2660	445	396
HCM Platoon Ratio	1.00	1.00	0.67	0.67	1.00	1.00
Upstream Filter(l)	1.00	1.00	0.28	0.28	1.00	1.00
Uniform Delay (d), s/veh	6.0	6.0	19.7	17.0	50.7	47.6
Incr Delay (d2), s/veh	1.3	1.2	0.3	0.3	8.2	1.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.0	6.3	1.0	16.7	6.5	2.5
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	7.2	7.2	20.0	17.3	58.9	48.8
LnGrp LOS	A	A	C	B	E	D
Approach Vol, veh/h	1320			1635	292	
Approach Delay, s/veh	7.2			17.4	55.8	
Approach LOS	A			B	E	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		96.8		23.2		96.8
Change Period (Y+Rc), s		7.0		7.0		7.0
Max Green Setting (Gmax), s		76.0		30.0		76.0
Max Q Clear Time (g_c+I1), s		19.3		15.4		40.1
Green Ext Time (p_c), s		13.3		0.8		18.6
Intersection Summary						
HCM 6th Ctrl Delay			16.7			
HCM 6th LOS			B			

Intersection												
Int Delay, s/veh	5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	0	0	0	0	61	0	26	19	22	10	0
Future Vol, veh/h	0	0	0	0	0	61	0	26	19	22	10	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	0	66	0	28	21	24	11	0

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	131	108	11	98	98	39	11	0	0	49	0	0
Stage 1	59	59	-	39	39	-	-	-	-	-	-	-
Stage 2	72	49	-	59	59	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	841	782	1070	884	792	1033	1608	-	-	1558	-	-
Stage 1	953	846	-	976	862	-	-	-	-	-	-	-
Stage 2	938	854	-	953	846	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	778	770	1070	874	780	1033	1608	-	-	1558	-	-
Mov Cap-2 Maneuver	778	770	-	874	780	-	-	-	-	-	-	-
Stage 1	953	833	-	976	862	-	-	-	-	-	-	-
Stage 2	878	854	-	939	833	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	0		8.7		0		5.1	
HCM LOS	A		A					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1608	-	-	-	-	1033	1558	-
HCM Lane V/C Ratio	-	-	-	-	-	0.064	0.015	-
HCM Control Delay (s)	0	-	-	0	8.7	7.3	0	-
HCM Lane LOS	A	-	-	A	A	A	A	-
HCM 95th %tile Q(veh)	0	-	-	-	0.2	0	-	-

Intersection						
Int Delay, s/veh	1.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↖	↗	↖	↗	↗	↖
Traffic Vol, veh/h	52	26	8	569	450	16
Future Vol, veh/h	52	26	8	569	450	16
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	50	175	-	-	135
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	57	28	9	618	489	17

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1125	489	506	0	-	0
Stage 1	489	-	-	-	-	-
Stage 2	636	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	227	579	1059	-	-	-
Stage 1	616	-	-	-	-	-
Stage 2	527	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	225	579	1059	-	-	-
Mov Cap-2 Maneuver	225	-	-	-	-	-
Stage 1	611	-	-	-	-	-
Stage 2	527	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	21.4	0.1	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	1059	-	225	579	-	-
HCM Lane V/C Ratio	0.008	-	0.251	0.049	-	-
HCM Control Delay (s)	8.4	-	26.3	11.5	-	-
HCM Lane LOS	A	-	D	B	-	-
HCM 95th %tile Q(veh)	0	-	1	0.2	-	-

Intersection						
Int Delay, s/veh	4.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↶	↷		↶	↷
Traffic Vol, veh/h	2	26	8	16	52	4
Future Vol, veh/h	2	26	8	16	52	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	2	28	9	17	57	4

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	26	0	-	0	50 18
Stage 1	-	-	-	-	18 -
Stage 2	-	-	-	-	32 -
Critical Hdwy	4.12	-	-	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	2.218	-	-	-	3.518 3.318
Pot Cap-1 Maneuver	1588	-	-	-	959 1061
Stage 1	-	-	-	-	1005 -
Stage 2	-	-	-	-	991 -
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	1588	-	-	-	958 1061
Mov Cap-2 Maneuver	-	-	-	-	958 -
Stage 1	-	-	-	-	1004 -
Stage 2	-	-	-	-	991 -

Approach	EB	WB	SB
HCM Control Delay, s	0.5	0	9
HCM LOS			A

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1588	-	-	-	965
HCM Lane V/C Ratio	0.001	-	-	-	0.063
HCM Control Delay (s)	7.3	0	-	-	9
HCM Lane LOS	A	A	-	-	A
HCM 95th %tile Q(veh)	0	-	-	-	0.2

Intersection												
Int Delay, s/veh	6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	1	2	0	0	4	8	0	0	0	26	0	4
Future Vol, veh/h	1	2	0	0	4	8	0	0	0	26	0	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	2	0	0	4	9	0	0	0	28	0	4

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	13	0	0	2	0	0	15	17	2	13	13	9
Stage 1	-	-	-	-	-	-	4	4	-	9	9	-
Stage 2	-	-	-	-	-	-	11	13	-	4	4	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1606	-	-	1620	-	-	1001	877	1082	1004	881	1073
Stage 1	-	-	-	-	-	-	1018	892	-	1012	888	-
Stage 2	-	-	-	-	-	-	1010	885	-	1018	892	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1606	-	-	1620	-	-	996	876	1082	1003	880	1073
Mov Cap-2 Maneuver	-	-	-	-	-	-	996	876	-	1003	880	-
Stage 1	-	-	-	-	-	-	1017	891	-	1011	888	-
Stage 2	-	-	-	-	-	-	1006	885	-	1017	891	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	2.4	0	0	8.7
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	1606	-	-	1620	-	-	1012
HCM Lane V/C Ratio	-	0.001	-	-	-	-	-	0.032
HCM Control Delay (s)	0	7.2	0	-	0	-	-	8.7
HCM Lane LOS	A	A	A	-	A	-	-	A
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	0.1

Timings
1: Chambers Rd & E. 104th Ave

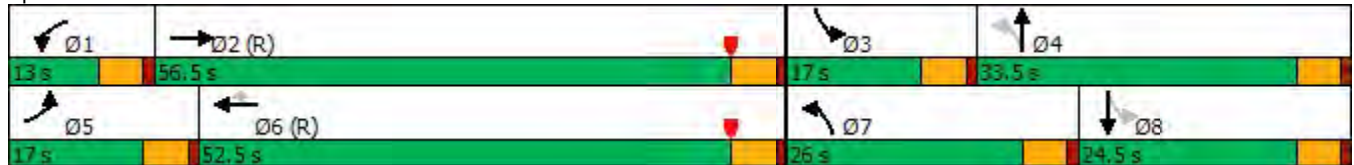
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Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	239	874	134	879	277	280	303	335	311
Future Volume (vph)	239	874	134	879	277	280	303	335	311
Turn Type	Prot	NA	Prot	NA	Perm	pm+pt	NA	pm+pt	NA
Protected Phases	5	2	1	6		7	4	3	8
Permitted Phases					6	4		8	
Detector Phase	5	2	1	6	6	7	4	3	8
Switch Phase									
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	11.0	24.0	11.0	24.5	24.5	11.0	24.0	11.0	24.5
Total Split (s)	17.0	56.5	13.0	52.5	52.5	26.0	33.5	17.0	24.5
Total Split (%)	14.2%	47.1%	10.8%	43.8%	43.8%	21.7%	27.9%	14.2%	20.4%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	C-Max	Max	C-Max	C-Max	Max	None	Max	Max
Act Effct Green (s)	12.0	51.5	8.0	47.5	47.5	45.5	28.5	31.5	19.5
Actuated g/C Ratio	0.10	0.43	0.07	0.40	0.40	0.38	0.24	0.26	0.16
v/c Ratio	0.80	0.86	0.76	0.82	0.43	0.83	0.58	0.63	0.81
Control Delay	82.5	29.7	76.6	38.1	4.1	50.3	38.7	32.6	57.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	82.5	29.7	76.6	38.1	4.1	50.3	38.7	32.6	57.1
LOS	F	C	E	D	A	D	D	C	E
Approach Delay		39.0		34.8			43.2		46.2
Approach LOS		D		C			D		D

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Yellow
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.86
 Intersection Signal Delay: 39.5
 Intersection LOS: D
 Intersection Capacity Utilization 80.2%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 1: Chambers Rd & E. 104th Ave



Queues
1: Chambers Rd & E. 104th Ave

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Chambers Rd Apartments Traffic Impact Study
































Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	275	1287	174	1142	360	311	489	381	470
v/c Ratio	0.80	0.86	0.76	0.82	0.43	0.83	0.58	0.63	0.81
Control Delay	82.5	29.7	76.6	38.1	4.1	50.3	38.7	32.6	57.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	82.5	29.7	76.6	38.1	4.1	50.3	38.7	32.6	57.1
Queue Length 50th (ft)	117	266	69	410	0	179	156	105	174
Queue Length 95th (ft)	#169	308	91	394	23	#329	213	140	#232
Internal Link Dist (ft)		2590		955			548		948
Turn Bay Length (ft)	275		380		350	290		285	
Base Capacity (vph)	343	1490	228	1400	844	373	844	603	580
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.80	0.86	0.76	0.82	0.43	0.83	0.58	0.63	0.81

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
 1: Chambers Rd & E. 104th Ave

Year 2027 w/Project PM
 Chambers Rd Apartments Traffic Impact Study

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	 		 	 			 		 	 	
Traffic Volume (veh/h)	239	874	245	134	879	277	280	303	137	335	311	103
Future Volume (veh/h)	239	874	245	134	879	277	280	303	137	335	311	103
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	275	1005	282	174	1142	0	311	337	152	381	353	117
Peak Hour Factor	0.87	0.87	0.87	0.77	0.77	0.77	0.90	0.90	0.90	0.88	0.88	0.88
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	346	1177	329	230	1407		400	569	252	659	428	140
Arrive On Green	0.20	0.86	0.86	0.07	0.40	0.00	0.17	0.24	0.24	0.10	0.16	0.16
Sat Flow, veh/h	3456	2743	766	3456	3554	1585	1781	2396	1060	3456	2633	860
Grp Volume(v), veh/h	275	649	638	174	1142	0	311	248	241	381	236	234
Grp Sat Flow(s),veh/h/ln	1728	1777	1732	1728	1777	1585	1781	1777	1679	1728	1777	1716
Q Serve(g_s), s	9.1	23.1	23.7	5.9	34.3	0.0	16.4	14.9	15.3	11.0	15.4	15.8
Cycle Q Clear(g_c), s	9.1	23.1	23.7	5.9	34.3	0.0	16.4	14.9	15.3	11.0	15.4	15.8
Prop In Lane	1.00		0.44	1.00		1.00	1.00		0.63	1.00		0.50
Lane Grp Cap(c), veh/h	346	763	744	230	1407		400	422	399	659	289	279
V/C Ratio(X)	0.80	0.85	0.86	0.76	0.81		0.78	0.59	0.60	0.58	0.82	0.84
Avail Cap(c_a), veh/h	346	763	744	230	1407		400	422	399	659	289	279
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	0.71	0.71	0.71	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	46.8	6.5	6.5	55.0	32.3	0.0	32.1	40.6	40.7	36.9	48.5	48.7
Incr Delay (d2), s/veh	12.7	8.5	9.1	20.4	5.2	0.0	13.8	2.1	2.6	3.7	22.2	24.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.1	4.5	4.6	3.2	15.0	0.0	8.4	6.6	6.5	4.9	8.5	8.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	59.5	15.0	15.6	75.4	37.5	0.0	45.9	42.7	43.3	40.6	70.7	73.6
LnGrp LOS	E	B	B	E	D		D	D	D	D	E	E
Approach Vol, veh/h		1562			1316			800			851	
Approach Delay, s/veh		23.1			42.5			44.1			58.0	
Approach LOS		C			D			D			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	13.0	56.5	17.0	33.5	17.0	52.5	26.0	24.5				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	8.0	51.5	12.0	28.5	12.0	47.5	21.0	19.5				
Max Q Clear Time (g_c+I1), s	7.9	25.7	13.0	17.3	11.1	36.3	18.4	17.8				
Green Ext Time (p_c), s	0.0	9.3	0.0	2.1	0.1	5.5	0.3	0.5				

Intersection Summary

HCM 6th Ctrl Delay	39.0
HCM 6th LOS	D

Notes

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

Timings
2: Chambers Rd & 96th Ave

Year 2027 w/Project PM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	332	749	569	199	1	197	1
Future Volume (vph)	332	749	569	199	1	197	1
Turn Type	Prot	NA	NA	Perm	NA	Split	NA
Protected Phases	5	2	6		8	4	4
Permitted Phases				6			
Detector Phase	5	2	6	6	8	4	4
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Total Split (s)	23.0	49.0	26.0	26.0	23.0	23.0	23.0
Total Split (%)	24.2%	51.6%	27.4%	27.4%	24.2%	24.2%	24.2%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?	Yes		Yes	Yes			
Recall Mode	None	Min	None	None	None	None	None
Act Effct Green (s)	18.2	44.4	21.2	21.2	5.7	13.7	13.7
Actuated g/C Ratio	0.26	0.63	0.30	0.30	0.08	0.20	0.20
v/c Ratio	0.75	0.65	1.06	0.33	0.03	0.66	0.34
Control Delay	37.8	13.3	84.0	5.4	28.5	36.3	7.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	37.8	13.3	84.0	5.4	28.5	36.3	7.6
LOS	D	B	F	A	C	D	A
Approach Delay		20.8	63.7		28.5		25.1
Approach LOS		C	E		C		C

Intersection Summary

Cycle Length: 95
 Actuated Cycle Length: 70.1
 Natural Cycle: 105
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 1.06
 Intersection Signal Delay: 36.6
 Intersection Capacity Utilization 99.4%
 Analysis Period (min) 15
 Intersection LOS: D
 ICU Level of Service F

Splits and Phases: 2: Chambers Rd & 96th Ave



Queues
2: Chambers Rd & 96th Ave

Year 2027 w/Project PM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	342	772	599	209	4	226	145
v/c Ratio	0.75	0.65	1.06	0.33	0.03	0.66	0.34
Control Delay	37.8	13.3	84.0	5.4	28.5	36.3	7.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	37.8	13.3	84.0	5.4	28.5	36.3	7.6
Queue Length 50th (ft)	129	164	~276	0	1	86	0
Queue Length 95th (ft)	#327	463	#599	51	6	172	42
Internal Link Dist (ft)		1966	2271		176		2572
Turn Bay Length (ft)	70			300			
Base Capacity (vph)	459	1180	563	624	451	459	517
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.75	0.65	1.06	0.33	0.01	0.49	0.28

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary

2: Chambers Rd & 96th Ave

Year 2027 w/Project PM
Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	332	749	0	0	569	199	0	1	1	197	1	125
Future Volume (veh/h)	332	749	0	0	569	199	0	1	1	197	1	125
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	342	772	0	0	599	209	0	2	2	226	1	144
Peak Hour Factor	0.97	0.97	0.97	0.95	0.95	0.95	0.50	0.50	0.50	0.87	0.87	0.87
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	393	1135	0	0	584	495	0	5	5	293	2	259
Arrive On Green	0.22	0.61	0.00	0.00	0.31	0.31	0.00	0.01	0.01	0.16	0.16	0.16
Sat Flow, veh/h	1781	1870	0	0	1870	1585	0	858	858	1781	11	1576
Grp Volume(v), veh/h	342	772	0	0	599	209	0	0	4	226	0	145
Grp Sat Flow(s),veh/h/ln	1781	1870	0	0	1870	1585	0	0	1716	1781	0	1587
Q Serve(g_s), s	12.5	18.6	0.0	0.0	21.0	7.0	0.0	0.0	0.2	8.2	0.0	5.7
Cycle Q Clear(g_c), s	12.5	18.6	0.0	0.0	21.0	7.0	0.0	0.0	0.2	8.2	0.0	5.7
Prop In Lane	1.00		0.00	0.00		1.00	0.00		0.50	1.00		0.99
Lane Grp Cap(c), veh/h	393	1135	0	0	584	495	0	0	9	293	0	261
V/C Ratio(X)	0.87	0.68	0.00	0.00	1.03	0.42	0.00	0.00	0.44	0.77	0.00	0.55
Avail Cap(c_a), veh/h	477	1223	0	0	584	495	0	0	459	477	0	424
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	25.3	8.8	0.0	0.0	23.1	18.3	0.0	0.0	33.4	26.9	0.0	25.8
Incr Delay (d2), s/veh	13.8	1.4	0.0	0.0	44.1	0.6	0.0	0.0	29.3	4.3	0.0	1.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.5	6.3	0.0	0.0	15.6	2.5	0.0	0.0	0.1	3.5	0.0	2.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	39.1	10.3	0.0	0.0	67.2	18.9	0.0	0.0	62.6	31.2	0.0	27.7
LnGrp LOS	D	B	A	A	F	B	A	A	E	C	A	C
Approach Vol, veh/h		1114			808			4				371
Approach Delay, s/veh		19.1			54.7			62.6				29.8
Approach LOS		B			D			E				C
Timer - Assigned Phs		2		4	5	6		8				
Phs Duration (G+Y+Rc), s		45.8		16.1	19.8	26.0		5.4				
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		5.0				
Max Green Setting (Gmax), s		44.0		18.0	18.0	21.0		18.0				
Max Q Clear Time (g_c+I1), s		20.6		10.2	14.5	23.0		2.2				
Green Ext Time (p_c), s		6.0		0.9	0.4	0.0		0.0				
Intersection Summary												
HCM 6th Ctrl Delay				33.4								
HCM 6th LOS				C								

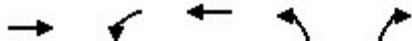
Intersection												
Int Delay, s/veh	11.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔		↔	↔		↔	↔		↔	↔	↔
Traffic Vol, veh/h	51	5	17	33	8	132	46	430	71	172	343	80
Future Vol, veh/h	51	5	17	33	8	132	46	430	71	172	343	80
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	150	-	-	100	-	-	235	-	-	235	-	135
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	55	5	18	36	9	143	50	467	77	187	373	87

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1429	1391	373	1408	1440	506	460	0	0	544	0	0
Stage 1	747	747	-	606	606	-	-	-	-	-	-	-
Stage 2	682	644	-	802	834	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	112	142	673	116	133	566	1101	-	-	1025	-	-
Stage 1	405	420	-	484	487	-	-	-	-	-	-	-
Stage 2	440	468	-	378	383	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	65	111	673	90	104	566	1101	-	-	1025	-	-
Mov Cap-2 Maneuver	65	111	-	90	104	-	-	-	-	-	-	-
Stage 1	387	344	-	462	465	-	-	-	-	-	-	-
Stage 2	308	447	-	296	313	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	128.8		27		0.7		2.7	
HCM LOS	F		D					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1101	-	-	65	313	90	451	1025	-	-
HCM Lane V/C Ratio	0.045	-	-	0.853	0.076	0.399	0.337	0.182	-	-
HCM Control Delay (s)	8.4	-	-	176.8	17.5	69.3	17	9.3	-	-
HCM Lane LOS	A	-	-	F	C	F	C	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	4	0.2	1.6	1.5	0.7	-	-

Timings
4: Sable Blvd & E. 104th Ave



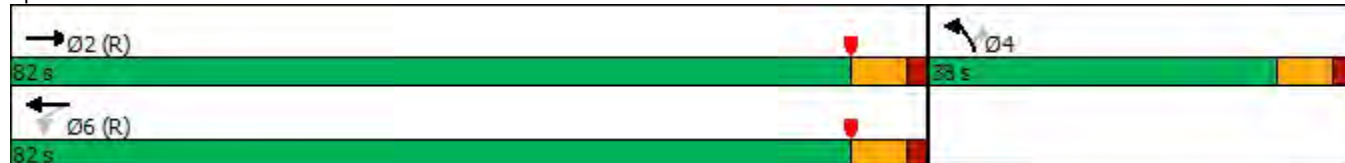
Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑	↑↑	↑	↑
Traffic Volume (vph)	1572	99	1341	98	61
Future Volume (vph)	1572	99	1341	98	61
Turn Type	NA	Perm	NA	Prot	Perm
Protected Phases	2		6	4	
Permitted Phases		6			4
Detector Phase	2	6	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	37.0	25.0	25.0	35.0	35.0
Total Split (s)	82.0	82.0	82.0	38.0	38.0
Total Split (%)	68.3%	68.3%	68.3%	31.7%	31.7%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	C-Max	C-Max	C-Max	None	None
Act Effct Green (s)	93.4	93.4	93.4	12.6	12.6
Actuated g/C Ratio	0.78	0.78	0.78	0.10	0.10
v/c Ratio	0.67	0.81	0.53	0.58	0.35
Control Delay	8.0	56.9	16.8	62.8	35.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	8.0	56.9	16.8	62.8	35.9
LOS	A	E	B	E	D
Approach Delay	8.0		19.6	52.5	
Approach LOS	A		B	D	

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Yellow
 Natural Cycle: 150
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.81
 Intersection Signal Delay: 15.3
 Intersection Capacity Utilization 75.2%
 Analysis Period (min) 15

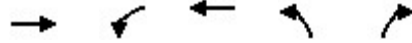
Intersection LOS: B
 ICU Level of Service D

Splits and Phases: 4: Sable Blvd & E. 104th Ave



Queues
4: Sable Blvd & E. 104th Ave

Year 2027 w/Project PM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1824	108	1458	107	66
v/c Ratio	0.67	0.81	0.53	0.58	0.35
Control Delay	8.0	56.9	16.8	62.8	35.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	8.0	56.9	16.8	62.8	35.9
Queue Length 50th (ft)	284	80	435	80	28
Queue Length 95th (ft)	425	m#99	561	134	70
Internal Link Dist (ft)	443		2590	1245	
Turn Bay Length (ft)		300		70	
Base Capacity (vph)	2732	134	2754	457	428
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.67	0.81	0.53	0.23	0.15

Intersection Summary

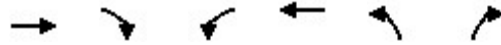
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
4: Sable Blvd & E. 104th Ave

Year 2027 w/Project PM
Chambers Rd Apartments Traffic Impact Study



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	↵
Traffic Volume (veh/h)	1572	106	99	1341	98	61
Future Volume (veh/h)	1572	106	99	1341	98	61
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	1709	115	108	1458	107	66
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	2719	181	214	2858	141	125
Arrive On Green	0.80	0.80	1.00	1.00	0.08	0.08
Sat Flow, veh/h	3475	226	256	3647	1781	1585
Grp Volume(v), veh/h	891	933	108	1458	107	66
Grp Sat Flow(s),veh/h/ln	1777	1830	256	1777	1781	1585
Q Serve(g_s), s	23.6	24.4	27.0	0.0	7.1	4.8
Cycle Q Clear(g_c), s	23.6	24.4	51.5	0.0	7.1	4.8
Prop In Lane		0.12	1.00		1.00	1.00
Lane Grp Cap(c), veh/h	1429	1471	214	2858	141	125
V/C Ratio(X)	0.62	0.63	0.51	0.51	0.76	0.53
Avail Cap(c_a), veh/h	1429	1471	214	2858	460	409
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	0.47	0.47	1.00	1.00
Uniform Delay (d), s/veh	4.6	4.7	6.5	0.0	54.1	53.1
Incr Delay (d2), s/veh	2.1	2.1	4.0	0.3	8.1	3.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.1	6.5	1.2	0.1	3.5	2.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	6.7	6.8	10.5	0.3	62.2	56.5
LnGrp LOS	A	A	B	A	E	E
Approach Vol, veh/h	1824			1566	173	
Approach Delay, s/veh	6.7			1.0	60.0	
Approach LOS	A			A	E	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		103.5		16.5		103.5
Change Period (Y+Rc), s		7.0		7.0		7.0
Max Green Setting (Gmax), s		75.0		31.0		75.0
Max Q Clear Time (g_c+I1), s		26.4		9.1		53.5
Green Ext Time (p_c), s		20.7		0.5		13.4
Intersection Summary						
HCM 6th Ctrl Delay			6.8			
HCM 6th LOS			A			

Intersection												
Int Delay, s/veh	5.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Vol, veh/h	0	0	0	0	0	43	0	19	0	70	31	0
Future Vol, veh/h	0	0	0	0	0	43	0	19	0	70	31	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	0	47	0	21	0	76	34	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	231	207	34	207	207	21	34	0	0	21	0	0
Stage 1	186	186	-	21	21	-	-	-	-	-	-	-
Stage 2	45	21	-	186	186	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	724	690	1039	751	690	1056	1578	-	-	1595	-	-
Stage 1	816	746	-	998	878	-	-	-	-	-	-	-
Stage 2	969	878	-	816	746	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	666	656	1039	723	656	1056	1578	-	-	1595	-	-
Mov Cap-2 Maneuver	666	656	-	723	656	-	-	-	-	-	-	-
Stage 1	816	709	-	998	878	-	-	-	-	-	-	-
Stage 2	926	878	-	776	709	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	0		8.6		0		5.1	
HCM LOS	A		A					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1578	-	-	-	-	1056	1595	-
HCM Lane V/C Ratio	-	-	-	-	0.044	0.048	-	-
HCM Control Delay (s)	0	-	-	0	8.6	7.4	0	-
HCM Lane LOS	A	-	-	A	A	A	A	-
HCM 95th %tile Q(veh)	0	-	-	-	0.1	0.1	-	-

Intersection						
Int Delay, s/veh	1.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↖	↗	↖	↗	↗	↖
Traffic Vol, veh/h	32	16	27	603	612	55
Future Vol, veh/h	32	16	27	603	612	55
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	50	175	-	-	135
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	35	17	29	655	665	60

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1378	665	725	0	-	0
Stage 1	665	-	-	-	-	-
Stage 2	713	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	160	460	878	-	-	-
Stage 1	511	-	-	-	-	-
Stage 2	486	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	155	460	878	-	-	-
Mov Cap-2 Maneuver	155	-	-	-	-	-
Stage 1	494	-	-	-	-	-
Stage 2	486	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	27.6	0.4	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	878	-	155	460	-	-
HCM Lane V/C Ratio	0.033	-	0.224	0.038	-	-
HCM Control Delay (s)	9.2	-	34.8	13.1	-	-
HCM Lane LOS	A	-	D	B	-	-
HCM 95th %tile Q(veh)	0.1	-	0.8	0.1	-	-

Intersection						
Int Delay, s/veh	2.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↶	↷		↶	↷
Traffic Vol, veh/h	5	16	27	55	32	3
Future Vol, veh/h	5	16	27	55	32	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	5	17	29	60	35	3
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	89	0	-	0	86	59
Stage 1	-	-	-	-	59	-
Stage 2	-	-	-	-	27	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1506	-	-	-	915	1007
Stage 1	-	-	-	-	964	-
Stage 2	-	-	-	-	996	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1506	-	-	-	912	1007
Mov Cap-2 Maneuver	-	-	-	-	912	-
Stage 1	-	-	-	-	961	-
Stage 2	-	-	-	-	996	-
Approach	EB	WB	SB			
HCM Control Delay, s	1.8	0	9.1			
HCM LOS			A			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1506	-	-	-	919	
HCM Lane V/C Ratio	0.004	-	-	-	0.041	
HCM Control Delay (s)	7.4	0	-	-	9.1	
HCM Lane LOS	A	A	-	-	A	
HCM 95th %tile Q(veh)	0	-	-	-	0.1	

Intersection												
Int Delay, s/veh	3.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	4	5	0	0	3	27	0	0	0	16	0	2
Future Vol, veh/h	4	5	0	0	3	27	0	0	0	16	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	4	5	0	0	3	29	0	0	0	17	0	2

Major/Minor	Major1		Major2			Minor1			Minor2			
Conflicting Flow All	32	0	0	5	0	0	32	45	5	31	31	18
Stage 1	-	-	-	-	-	-	13	13	-	18	18	-
Stage 2	-	-	-	-	-	-	19	32	-	13	13	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1580	-	-	1616	-	-	976	847	1078	977	862	1061
Stage 1	-	-	-	-	-	-	1007	885	-	1001	880	-
Stage 2	-	-	-	-	-	-	1000	868	-	1007	885	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1580	-	-	1616	-	-	972	844	1078	975	859	1061
Mov Cap-2 Maneuver	-	-	-	-	-	-	972	844	-	975	859	-
Stage 1	-	-	-	-	-	-	1004	882	-	998	880	-
Stage 2	-	-	-	-	-	-	998	868	-	1004	882	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	3.2	0	0	8.7
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	1580	-	-	1616	-	-	984
HCM Lane V/C Ratio	-	0.003	-	-	-	-	-	0.02
HCM Control Delay (s)	0	7.3	0	-	0	-	-	8.7
HCM Lane LOS	A	A	A	-	A	-	-	A
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	0.1

Timings
1: Chambers Rd & E. 104th Ave

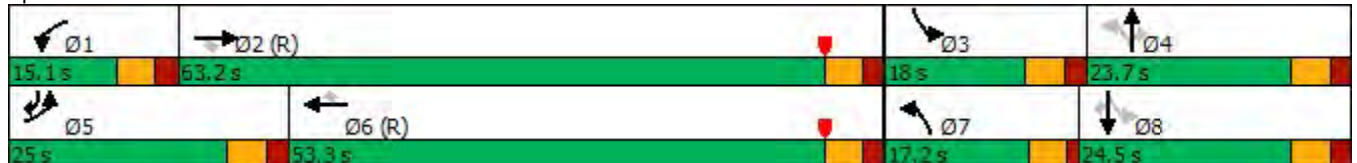
Year 2044 Background AM
Chambers Rd Apartments Traffic Impact Study

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	160	1120	205	130	1480	295	375	275	160	345	230	315
Future Volume (vph)	160	1120	205	130	1480	295	375	275	160	345	230	315
Turn Type	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	pm+ov
Protected Phases	5	2		1	6		7	4		3	8	5
Permitted Phases			2			6	4		4	8		8
Detector Phase	5	2	2	1	6	6	7	4	4	3	8	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.5	23.5	23.5	10.5	24.5	24.5	9.5	23.5	23.5	10.5	24.5	10.5
Total Split (s)	25.0	63.2	63.2	15.1	53.3	53.3	17.2	23.7	23.7	18.0	24.5	25.0
Total Split (%)	20.8%	52.7%	52.7%	12.6%	44.4%	44.4%	14.3%	19.8%	19.8%	15.0%	20.4%	20.8%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.5	5.5	5.5	5.5	5.5	5.5	4.5	5.5	5.5	5.5	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	None
Act Effct Green (s)	12.1	61.0	61.0	9.3	58.1	58.1	29.0	15.3	15.3	28.5	16.1	33.7
Actuated g/C Ratio	0.10	0.51	0.51	0.08	0.48	0.48	0.24	0.13	0.13	0.24	0.13	0.28
v/c Ratio	0.50	0.47	0.24	0.53	0.65	0.34	0.66	0.66	0.49	0.69	0.53	0.70
Control Delay	60.0	13.3	2.6	60.9	25.9	3.4	37.2	53.2	10.6	42.5	52.2	38.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	60.0	13.3	2.6	60.9	25.9	3.4	37.2	53.2	10.6	42.5	52.2	38.3
LOS	E	B	A	E	C	A	D	D	B	D	D	D
Approach Delay		16.9			24.8			37.4			43.5	
Approach LOS		B			C			D			D	

Intersection Summary

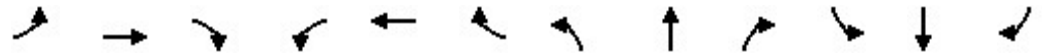
Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 44.6 (37%), Referenced to phase 2:EBT and 6:WBT, Start of Yellow
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.70
 Intersection Signal Delay: 27.8
 Intersection Capacity Utilization 71.7%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service C

Splits and Phases: 1: Chambers Rd & E. 104th Ave



Queues
1: Chambers Rd & E. 104th Ave

Year 2044 Background AM
Chambers Rd Apartments Traffic Impact Study




































Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	174	1217	223	141	1609	321	408	299	174	375	250	342
v/c Ratio	0.50	0.47	0.24	0.53	0.65	0.34	0.66	0.66	0.49	0.69	0.53	0.70
Control Delay	60.0	13.3	2.6	60.9	25.9	3.4	37.2	53.2	10.6	42.5	52.2	38.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	60.0	13.3	2.6	60.9	25.9	3.4	37.2	53.2	10.6	42.5	52.2	38.3
Queue Length 50th (ft)	55	230	33	54	331	0	116	105	3	121	96	196
Queue Length 95th (ft)	80	256	33	89	449	55	170	157	61	160	136	271
Internal Link Dist (ft)		3380			955			548			948	
Turn Bay Length (ft)	275		250	380		350	290		250	285		250
Base Capacity (vph)	557	2583	913	280	2462	932	620	536	387	542	560	584
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.31	0.47	0.24	0.50	0.65	0.34	0.66	0.56	0.45	0.69	0.45	0.59

Intersection Summary

HCM 6th Signalized Intersection Summary
 1: Chambers Rd & E. 104th Ave

Year 2024 Background AM
 Chambers Rd Apartments Traffic Impact Study

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	  		 	  		 	 		 	 	
Traffic Volume (veh/h)	160	1120	205	130	1480	295	375	275	160	345	230	315
Future Volume (veh/h)	160	1120	205	130	1480	295	375	275	160	345	230	315
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	174	1217	223	141	1609	0	408	299	174	375	250	342
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	238	2572	799	197	2511		637	539	240	612	563	360
Arrive On Green	0.07	0.50	0.50	0.06	0.49	0.00	0.11	0.15	0.15	0.10	0.16	0.16
Sat Flow, veh/h	3456	5106	1585	3456	5106	1585	3456	3554	1585	3456	3554	1585
Grp Volume(v), veh/h	174	1217	223	141	1609	0	408	299	174	375	250	342
Grp Sat Flow(s),veh/h/ln	1728	1702	1585	1728	1702	1585	1728	1777	1585	1728	1777	1585
Q Serve(g_s), s	5.9	18.6	9.7	4.8	28.1	0.0	11.9	9.4	12.6	10.9	7.6	19.0
Cycle Q Clear(g_c), s	5.9	18.6	9.7	4.8	28.1	0.0	11.9	9.4	12.6	10.9	7.6	19.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	238	2572	799	197	2511		637	539	240	612	563	360
V/C Ratio(X)	0.73	0.47	0.28	0.72	0.64		0.64	0.55	0.72	0.61	0.44	0.95
Avail Cap(c_a), veh/h	562	2572	799	276	2511		637	539	240	612	563	360
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.85	0.85	0.85	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	54.8	19.4	17.2	55.6	22.6	0.0	37.8	47.1	48.5	37.7	45.7	45.7
Incr Delay (d2), s/veh	3.6	0.5	0.7	5.1	1.3	0.0	2.2	1.2	10.3	1.8	0.6	34.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.6	7.0	3.6	2.2	10.8	0.0	5.1	4.2	5.6	4.7	3.4	13.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	58.4	19.9	17.9	60.7	23.9	0.0	39.9	48.4	58.8	39.5	46.3	80.0
LnGrp LOS	E	B	B	E	C		D	D	E	D	D	E
Approach Vol, veh/h		1614			1750			881			967	
Approach Delay, s/veh		23.8			26.9			46.5			55.6	
Approach LOS		C			C			D			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.3	66.0	18.0	23.7	13.8	64.5	17.2	24.5				
Change Period (Y+Rc), s	5.5	5.5	5.5	5.5	5.5	5.5	4.5	5.5				
Max Green Setting (Gmax), s	9.6	57.7	12.5	18.2	19.5	47.8	12.7	19.0				
Max Q Clear Time (g_c+I1), s	6.8	20.6	12.9	14.6	7.9	30.1	13.9	21.0				
Green Ext Time (p_c), s	0.1	11.2	0.0	0.8	0.4	10.2	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			34.6									
HCM 6th LOS			C									
Notes												
User approved pedestrian interval to be less than phase max green.												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Timings
2: Chambers Rd & 96th Ave

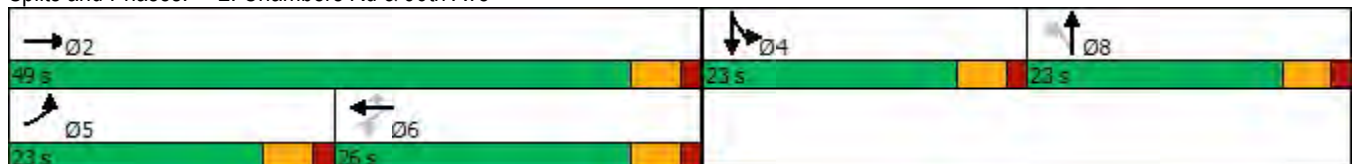


Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	110	410	895	215	0	270	3
Future Volume (vph)	110	410	895	215	0	270	3
Turn Type	Prot	NA	NA	Perm	NA	Split	NA
Protected Phases	5	2	6		8	4	4
Permitted Phases				6			
Detector Phase	5	2	6	6	8	4	4
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Total Split (s)	23.0	49.0	26.0	26.0	23.0	23.0	23.0
Total Split (%)	24.2%	51.6%	27.4%	27.4%	24.2%	24.2%	24.2%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?	Yes		Yes	Yes			
Recall Mode	None	Min	None	None	None	None	None
Act Effct Green (s)	9.8	33.5	21.8	21.8	5.7	16.4	16.4
Actuated g/C Ratio	0.16	0.54	0.35	0.35	0.09	0.26	0.26
v/c Ratio	0.43	0.23	0.78	0.33	0.03	0.63	0.50
Control Delay	31.2	8.3	27.1	4.9	0.2	29.8	6.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	31.2	8.3	27.1	4.9	0.2	29.8	6.3
LOS	C	A	C	A	A	C	A
Approach Delay		13.1	22.8		0.2		17.3
Approach LOS		B	C		A		B

Intersection Summary

Cycle Length: 95
 Actuated Cycle Length: 61.9
 Natural Cycle: 95
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 0.78
 Intersection Signal Delay: 19.0
 Intersection Capacity Utilization 70.3%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service C

Splits and Phases: 2: Chambers Rd & 96th Ave



Queues
2: Chambers Rd & 96th Ave



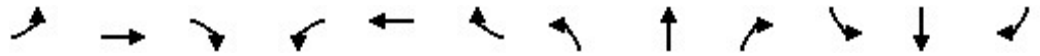
Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	120	448	973	234	10	293	329
v/c Ratio	0.43	0.23	0.78	0.33	0.03	0.63	0.50
Control Delay	31.2	8.3	27.1	4.9	0.2	29.8	6.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	31.2	8.3	27.1	4.9	0.2	29.8	6.3
Queue Length 50th (ft)	43	41	179	0	0	97	1
Queue Length 95th (ft)	101	88	#392	51	0	#242	64
Internal Link Dist (ft)		1966	2271		176		2689
Turn Bay Length (ft)	425			300			
Base Capacity (vph)	534	2594	1247	709	667	534	706
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.22	0.17	0.78	0.33	0.01	0.55	0.47

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
 2: Chambers Rd & 96th Ave

Year 2044 Background AM
 Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	110	410	2	0	895	215	0	0	5	270	3	300
Future Volume (veh/h)	110	410	2	0	895	215	0	0	5	270	3	300
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	120	446	2	0	973	234	0	0	10	293	3	326
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.50	0.50	0.50	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	179	1816	8	0	1138	507	0	0	20	439	4	388
Arrive On Green	0.10	0.50	0.50	0.00	0.32	0.32	0.00	0.00	0.01	0.25	0.25	0.25
Sat Flow, veh/h	1781	3628	16	0	3647	1585	0	0	1585	1781	14	1573
Grp Volume(v), veh/h	120	218	230	0	973	234	0	0	10	293	0	329
Grp Sat Flow(s),veh/h/ln	1781	1777	1867	0	1777	1585	0	0	1585	1781	0	1587
Q Serve(g_s), s	4.1	4.4	4.4	0.0	16.0	7.4	0.0	0.0	0.4	9.3	0.0	12.3
Cycle Q Clear(g_c), s	4.1	4.4	4.4	0.0	16.0	7.4	0.0	0.0	0.4	9.3	0.0	12.3
Prop In Lane	1.00		0.01	0.00		1.00	0.00		1.00	1.00		0.99
Lane Grp Cap(c), veh/h	179	889	935	0	1138	507	0	0	20	439	0	391
V/C Ratio(X)	0.67	0.25	0.25	0.00	0.86	0.46	0.00	0.00	0.49	0.67	0.00	0.84
Avail Cap(c_a), veh/h	513	1252	1316	0	1195	533	0	0	457	513	0	457
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	27.1	8.9	8.9	0.0	19.9	16.9	0.0	0.0	30.6	21.2	0.0	22.4
Incr Delay (d2), s/veh	4.3	0.1	0.1	0.0	6.1	0.7	0.0	0.0	17.5	2.6	0.0	11.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.9	1.5	1.5	0.0	6.9	2.5	0.0	0.0	0.2	3.7	0.0	5.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	31.4	9.0	9.0	0.0	25.9	17.6	0.0	0.0	48.1	23.9	0.0	34.1
LnGrp LOS	C	A	A	A	C	B	A	A	D	C	A	C
Approach Vol, veh/h		568			1207			10			622	
Approach Delay, s/veh		13.8			24.3			48.1			29.3	
Approach LOS		B			C			D			C	
Timer - Assigned Phs		2		4	5	6		8				
Phs Duration (G+Y+Rc), s		36.3		20.4	11.3	25.0		5.8				
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		5.0				
Max Green Setting (Gmax), s		44.0		18.0	18.0	21.0		18.0				
Max Q Clear Time (g_c+I1), s		6.4		14.3	6.1	18.0		2.4				
Green Ext Time (p_c), s		2.9		1.1	0.2	2.0		0.0				
Intersection Summary												
HCM 6th Ctrl Delay				23.2								
HCM 6th LOS				C								

Timings
3: Chambers Rd & 100th Ave

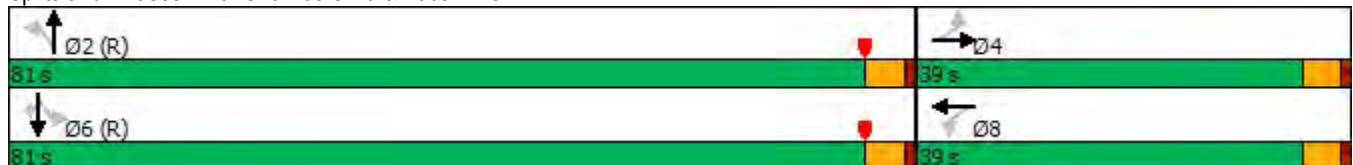


Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Configurations	↖	↗	↖	↗	↖	↗	↖	↗	↖
Traffic Volume (vph)	75	15	90	12	15	415	180	415	30
Future Volume (vph)	75	15	90	12	15	415	180	415	30
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases		4		8		2		6	
Permitted Phases	4		8		2		6		6
Detector Phase	4	4	8	8	2	2	6	6	6
Switch Phase									
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	39.0	39.0	39.0	39.0	81.0	81.0	81.0	81.0	81.0
Total Split (%)	32.5%	32.5%	32.5%	32.5%	67.5%	67.5%	67.5%	67.5%	67.5%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	16.8	16.8	16.8	16.8	94.2	94.2	94.2	94.2	94.2
Actuated g/C Ratio	0.14	0.14	0.14	0.14	0.78	0.78	0.78	0.78	0.78
v/c Ratio	1.32	0.22	0.52	0.57	0.02	0.35	0.30	0.31	0.03
Control Delay	266.0	18.9	56.2	12.3	4.3	5.2	17.1	15.8	7.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	266.0	18.9	56.2	12.3	4.3	5.2	17.1	15.8	7.6
LOS	F	B	E	B	A	A	B	B	A
Approach Delay		162.6		25.2		5.2		15.8	
Approach LOS		F		C		A		B	

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow
 Natural Cycle: 55
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.32
 Intersection Signal Delay: 26.7
 Intersection Capacity Utilization 67.3%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service C

Splits and Phases: 3: Chambers Rd & 100th Ave



Queues
3: Chambers Rd & 100th Ave



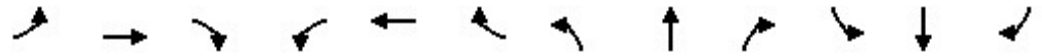
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	82	59	98	236	16	505	196	451	33
v/c Ratio	1.32	0.22	0.52	0.57	0.02	0.35	0.30	0.31	0.03
Control Delay	266.0	18.9	56.2	12.3	4.3	5.2	17.1	15.8	7.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	266.0	18.9	56.2	12.3	4.3	5.2	17.1	15.8	7.6
Queue Length 50th (ft)	~82	11	71	9	2	93	92	246	2
Queue Length 95th (ft)	#162	46	118	77	10	190	201	394	m25
Internal Link Dist (ft)		1101		1411		2689		1855	
Turn Bay Length (ft)	150		100		235		235		135
Base Capacity (vph)	127	507	384	618	701	1441	657	1462	1249
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.65	0.12	0.26	0.38	0.02	0.35	0.30	0.31	0.03

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

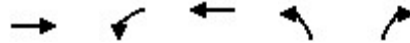
HCM 6th Signalized Intersection Summary
 3: Chambers Rd & 100th Ave

Year 2044 Background AM
 Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	75	15	40	90	12	205	15	415	50	180	415	30
Future Volume (veh/h)	75	15	40	90	12	205	15	415	50	180	415	30
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	82	16	43	98	13	223	16	451	54	196	451	33
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	163	100	270	323	20	338	629	1149	138	585	1311	1111
Arrive On Green	0.22	0.22	0.22	0.22	0.22	0.22	0.70	0.70	0.70	0.70	0.70	0.70
Sat Flow, veh/h	1144	448	1205	1344	88	1510	940	1639	196	894	1870	1585
Grp Volume(v), veh/h	82	0	59	98	0	236	16	0	505	196	451	33
Grp Sat Flow(s),veh/h/ln	1144	0	1653	1344	0	1598	940	0	1835	894	1870	1585
Q Serve(g_s), s	8.4	0.0	3.4	7.6	0.0	16.1	0.8	0.0	13.6	13.9	11.4	0.8
Cycle Q Clear(g_c), s	24.6	0.0	3.4	11.0	0.0	16.1	12.2	0.0	13.6	27.5	11.4	0.8
Prop In Lane	1.00		0.73	1.00		0.94	1.00		0.11	1.00		1.00
Lane Grp Cap(c), veh/h	163	0	370	323	0	358	629	0	1286	585	1311	1111
V/C Ratio(X)	0.50	0.00	0.16	0.30	0.00	0.66	0.03	0.00	0.39	0.33	0.34	0.03
Avail Cap(c_a), veh/h	235	0	475	408	0	460	629	0	1286	585	1311	1111
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	0.00	1.00	0.94	0.00	0.94	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.6	0.0	37.5	41.9	0.0	42.4	9.5	0.0	7.4	13.1	7.1	5.5
Incr Delay (d2), s/veh	2.4	0.0	0.2	0.5	0.0	2.3	0.1	0.0	0.8	1.5	0.7	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.6	0.0	1.4	2.6	0.0	6.7	0.2	0.0	5.2	3.0	4.5	0.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	56.0	0.0	37.7	42.4	0.0	44.7	9.6	0.0	8.3	14.6	7.8	5.5
LnGrp LOS	E	A	D	D	A	D	A	A	A	B	A	A
Approach Vol, veh/h		141			334			521			680	
Approach Delay, s/veh		48.3			44.0			8.3			9.7	
Approach LOS		D			D			A			A	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		88.6		31.4		88.6		31.4				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		76.5		34.5		76.5		34.5				
Max Q Clear Time (g_c+I1), s		15.6		26.6		29.5		18.1				
Green Ext Time (p_c), s		3.9		0.3		4.8		1.7				
Intersection Summary												
HCM 6th Ctrl Delay				19.3								
HCM 6th LOS				B								

Timings
4: Sable Blvd & E. 104th Ave

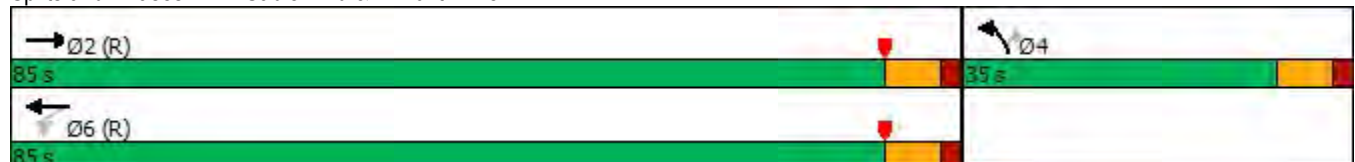


Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Configurations	↑↑↑	↙	↑↑↑	↙	↗
Traffic Volume (vph)	1660	65	2015	230	110
Future Volume (vph)	1660	65	2015	230	110
Turn Type	NA	Perm	NA	Prot	Perm
Protected Phases	2		6	4	
Permitted Phases		6			4
Detector Phase	2	6	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	37.0	25.0	25.0	35.0	35.0
Total Split (s)	85.0	85.0	85.0	35.0	35.0
Total Split (%)	70.8%	70.8%	70.8%	29.2%	29.2%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	C-Min	C-Max	C-Max	None	None
Act Effct Green (s)	84.2	84.2	84.2	21.8	21.8
Actuated g/C Ratio	0.70	0.70	0.70	0.18	0.18
v/c Ratio	0.52	0.59	0.61	0.78	0.39
Control Delay	9.5	27.6	6.9	62.9	36.5
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	9.5	27.6	6.9	62.9	36.5
LOS	A	C	A	E	D
Approach Delay	9.5		7.5	54.3	
Approach LOS	A		A	D	

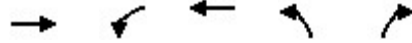
Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Yellow
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.78
 Intersection Signal Delay: 12.2
 Intersection LOS: B
 Intersection Capacity Utilization 67.3%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 4: Sable Blvd & E. 104th Ave



Queues
4: Sable Blvd & E. 104th Ave



Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1842	71	2190	250	120
v/c Ratio	0.52	0.59	0.61	0.78	0.39
Control Delay	9.5	27.6	6.9	62.9	36.5
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	9.5	27.6	6.9	62.9	36.5
Queue Length 50th (ft)	220	18	205	186	64
Queue Length 95th (ft)	303	m37	219	263	116
Internal Link Dist (ft)	529		3380	1798	
Turn Bay Length (ft)		300		125	
Base Capacity (vph)	3558	121	3567	413	388
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.52	0.59	0.61	0.61	0.31

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
4: Sable Blvd & E. 104th Ave

Year 2044 Background AM
Chambers Rd Apartments Traffic Impact Study



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑↑		↵	↑↑↑	↵	↵
Traffic Volume (veh/h)	1660	35	65	2015	230	110
Future Volume (veh/h)	1660	35	65	2015	230	110
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	1804	38	71	2190	250	120
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	3719	78	204	3689	286	255
Arrive On Green	0.72	0.72	0.72	0.72	0.16	0.16
Sat Flow, veh/h	5315	108	251	5274	1781	1585
Grp Volume(v), veh/h	1193	649	71	2190	250	120
Grp Sat Flow(s),veh/h/ln	1702	1851	251	1702	1781	1585
Q Serve(g_s), s	18.0	18.0	20.2	25.0	16.4	8.2
Cycle Q Clear(g_c), s	18.0	18.0	38.2	25.0	16.4	8.2
Prop In Lane		0.06	1.00		1.00	1.00
Lane Grp Cap(c), veh/h	2460	1337	204	3689	286	255
V/C Ratio(X)	0.48	0.49	0.35	0.59	0.87	0.47
Avail Cap(c_a), veh/h	2460	1337	204	3689	416	370
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	0.70	0.70	1.00	1.00
Uniform Delay (d), s/veh	7.1	7.1	15.3	8.1	49.2	45.7
Incr Delay (d2), s/veh	0.7	1.3	3.3	0.5	13.2	1.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.1	6.8	1.3	8.3	8.4	3.4
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	7.8	8.4	18.5	8.6	62.3	47.1
LnGrp LOS	A	A	B	A	E	D
Approach Vol, veh/h	1842			2261	370	
Approach Delay, s/veh	8.0			8.9	57.4	
Approach LOS	A			A	E	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		93.7		26.3		93.7
Change Period (Y+Rc), s		7.0		7.0		7.0
Max Green Setting (Gmax), s		78.0		28.0		78.0
Max Q Clear Time (g_c+I1), s		20.0		18.4		40.2
Green Ext Time (p_c), s		23.4		0.9		28.2
Intersection Summary						
HCM 6th Ctrl Delay			12.5			
HCM 6th LOS			B			

Intersection												
Int Delay, s/veh	5.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	0	0	0	0	55	0	25	0	20	10	0
Future Vol, veh/h	0	0	0	0	0	55	0	25	0	20	10	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	0	60	0	27	0	22	11	0

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	112	82	11	82	82	27	11	0	0	27	0	0
Stage 1	55	55	-	27	27	-	-	-	-	-	-	-
Stage 2	57	27	-	55	55	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	866	808	1070	905	808	1048	1608	-	-	1587	-	-
Stage 1	957	849	-	990	873	-	-	-	-	-	-	-
Stage 2	955	873	-	957	849	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	808	797	1070	895	797	1048	1608	-	-	1587	-	-
Mov Cap-2 Maneuver	808	797	-	895	797	-	-	-	-	-	-	-
Stage 1	957	837	-	990	873	-	-	-	-	-	-	-
Stage 2	901	873	-	944	837	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	0		8.6		0		4.9	
HCM LOS	A		A					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1608	-	-	-	1048	1587	-
HCM Lane V/C Ratio	-	-	-	-	0.057	0.014	-
HCM Control Delay (s)	0	-	-	0	8.6	7.3	0
HCM Lane LOS	A	-	-	A	A	A	A
HCM 95th %tile Q(veh)	0	-	-	-	0.2	0	-

Timings
1: Chambers Rd & E. 104th Ave

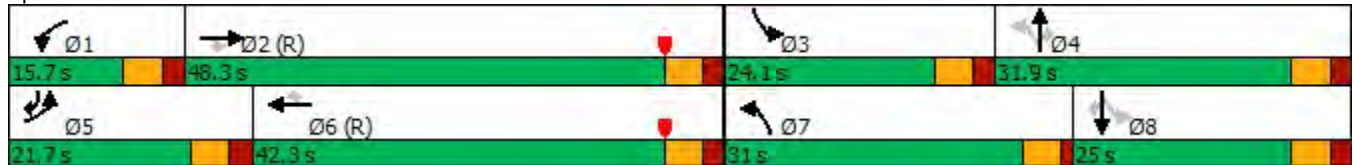
Year 2044 Background PM
Chambers Rd Apartments Traffic Impact Study

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	340	1240	285	145	1245	395	360	425	170	475	430	145
Future Volume (vph)	340	1240	285	145	1245	395	360	425	170	475	430	145
Turn Type	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	pm+ov
Protected Phases	5	2		1	6		7	4		3	8	5
Permitted Phases			2			6	4		4	8		8
Detector Phase	5	2	2	1	6	6	7	4	4	3	8	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.5	23.5	23.5	10.5	24.5	24.5	9.5	23.5	23.5	10.5	24.5	10.5
Total Split (s)	21.7	48.3	48.3	15.7	42.3	42.3	31.0	31.9	31.9	24.1	25.0	21.7
Total Split (%)	18.1%	40.3%	40.3%	13.1%	35.3%	35.3%	25.8%	26.6%	26.6%	20.1%	20.8%	18.1%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.5	5.5	5.5	5.5	5.5	5.5	4.5	5.5	5.5	5.5	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	None
Act Effct Green (s)	16.6	48.5	48.5	10.1	42.0	42.0	37.6	21.6	21.6	43.1	25.4	47.5
Actuated g/C Ratio	0.14	0.40	0.40	0.08	0.35	0.35	0.31	0.18	0.18	0.36	0.21	0.40
v/c Ratio	0.78	0.66	0.38	0.55	0.76	0.52	0.58	0.73	0.43	0.76	0.62	0.23
Control Delay	71.6	26.5	3.3	60.0	39.0	5.4	29.4	53.0	9.5	35.0	46.7	8.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	71.6	26.5	3.3	60.0	39.0	5.4	29.4	53.0	9.5	35.0	46.7	8.0
LOS	E	C	A	E	D	A	C	D	A	D	D	A
Approach Delay		31.2			33.2			36.3			36.1	
Approach LOS		C			C			D			D	

Intersection Summary

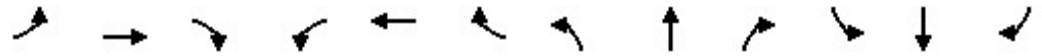
Cycle Length: 120	
Actuated Cycle Length: 120	
Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Yellow	
Natural Cycle: 90	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.78	
Intersection Signal Delay: 33.6	Intersection LOS: C
Intersection Capacity Utilization 77.4%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 1: Chambers Rd & E. 104th Ave



Queues
1: Chambers Rd & E. 104th Ave

Year 2044 Background PM
Chambers Rd Apartments Traffic Impact Study





































Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	370	1348	310	158	1353	429	391	462	185	516	467	158
v/c Ratio	0.78	0.66	0.38	0.55	0.76	0.52	0.58	0.73	0.43	0.76	0.62	0.23
Control Delay	71.6	26.5	3.3	60.0	39.0	5.4	29.4	53.0	9.5	35.0	46.7	8.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	71.6	26.5	3.3	60.0	39.0	5.4	29.4	53.0	9.5	35.0	46.7	8.0
Queue Length 50th (ft)	156	213	0	61	350	0	106	179	3	148	172	20
Queue Length 95th (ft)	#221	283	48	97	424	76	135	226	63	181	225	64
Internal Link Dist (ft)		3380			955			548			948	
Turn Bay Length (ft)	275		250	380		350	290		250	285		250
Base Capacity (vph)	489	2056	825	305	1780	832	965	778	488	702	748	701
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.76	0.66	0.38	0.52	0.76	0.52	0.41	0.59	0.38	0.74	0.62	0.23

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
 1: Chambers Rd & E. 104th Ave

Year 2044 Background PM
 Chambers Rd Apartments Traffic Impact Study

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	  		 	  		 	 		 	 	
Traffic Volume (veh/h)	340	1240	285	145	1245	395	360	425	170	475	430	145
Future Volume (veh/h)	340	1240	285	145	1245	395	360	425	170	475	430	145
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	370	1348	310	158	1353	0	391	462	185	516	467	158
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	427	2300	714	215	1986		640	576	257	666	690	504
Arrive On Green	0.12	0.45	0.45	0.06	0.39	0.00	0.12	0.16	0.16	0.14	0.19	0.19
Sat Flow, veh/h	3456	5106	1585	3456	5106	1585	3456	3554	1585	3456	3554	1585
Grp Volume(v), veh/h	370	1348	310	158	1353	0	391	462	185	516	467	158
Grp Sat Flow(s),veh/h/ln	1728	1702	1585	1728	1702	1585	1728	1777	1585	1728	1777	1585
Q Serve(g_s), s	12.6	23.7	16.0	5.4	26.4	0.0	11.0	15.0	13.3	14.6	14.6	9.1
Cycle Q Clear(g_c), s	12.6	23.7	16.0	5.4	26.4	0.0	11.0	15.0	13.3	14.6	14.6	9.1
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	427	2300	714	215	1986		640	576	257	666	690	504
V/C Ratio(X)	0.87	0.59	0.43	0.73	0.68		0.61	0.80	0.72	0.77	0.68	0.31
Avail Cap(c_a), veh/h	467	2300	714	294	1986		995	782	349	712	690	504
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.74	0.74	0.74	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	51.6	24.6	22.5	55.3	30.5	0.0	35.7	48.4	47.7	35.3	44.8	31.0
Incr Delay (d2), s/veh	11.4	0.8	1.4	6.1	1.9	0.0	0.9	4.3	4.6	5.0	2.6	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.0	9.2	6.1	2.5	10.7	0.0	4.6	6.9	5.5	6.4	6.6	3.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	63.0	25.4	23.9	61.4	32.4	0.0	36.7	52.8	52.3	40.3	47.5	31.4
LnGrp LOS	E	C	C	E	C		D	D	D	D	D	C
Approach Vol, veh/h		2028			1511			1038			1141	
Approach Delay, s/veh		32.1			35.4			46.6			42.0	
Approach LOS		C			D			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	13.0	59.6	22.5	24.9	20.3	52.2	18.7	28.8				
Change Period (Y+Rc), s	5.5	5.5	5.5	5.5	5.5	5.5	4.5	5.5				
Max Green Setting (Gmax), s	10.2	42.8	18.6	26.4	16.2	36.8	26.5	19.5				
Max Q Clear Time (g_c+I1), s	7.4	25.7	16.6	17.0	14.6	28.4	13.0	16.6				
Green Ext Time (p_c), s	0.1	9.3	0.4	2.4	0.2	5.1	1.1	1.0				
Intersection Summary												
HCM 6th Ctrl Delay			37.6									
HCM 6th LOS			D									
Notes												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Timings
2: Chambers Rd & 96th Ave



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	415	1065	810	255	2	265	2
Future Volume (vph)	415	1065	810	255	2	265	2
Turn Type	Prot	NA	NA	Perm	NA	Split	NA
Protected Phases	5	2	6		8	4	4
Permitted Phases				6			
Detector Phase	5	2	6	6	8	4	4
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Total Split (s)	23.0	49.0	26.0	26.0	23.0	23.0	23.0
Total Split (%)	24.2%	51.6%	27.4%	27.4%	24.2%	24.2%	24.2%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?	Yes		Yes	Yes			
Recall Mode	None	Min	None	None	None	None	None
Act Effct Green (s)	18.1	44.3	21.1	21.1	5.9	16.5	16.5
Actuated g/C Ratio	0.25	0.61	0.29	0.29	0.08	0.23	0.23
v/c Ratio	0.97	0.51	0.83	0.41	0.06	0.72	0.33
Control Delay	68.3	10.1	34.2	5.5	27.5	38.5	7.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	68.3	10.1	34.2	5.5	27.5	38.5	7.0
LOS	E	B	C	A	C	D	A
Approach Delay		26.4	27.4		27.5		27.2
Approach LOS		C	C		C		C

Intersection Summary

Cycle Length: 95
 Actuated Cycle Length: 72.9
 Natural Cycle: 105
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 0.97
 Intersection Signal Delay: 26.9
 Intersection Capacity Utilization 85.7%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service E

Splits and Phases: 2: Chambers Rd & 96th Ave



Queues
2: Chambers Rd & 96th Ave



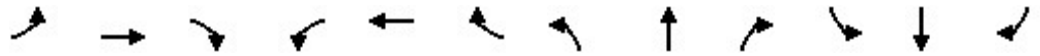
Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	428	1098	853	268	8	288	160
v/c Ratio	0.97	0.51	0.83	0.41	0.06	0.72	0.33
Control Delay	68.3	10.1	34.2	5.5	27.5	38.5	7.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	68.3	10.1	34.2	5.5	27.5	38.5	7.0
Queue Length 50th (ft)	188	128	184	0	2	115	1
Queue Length 95th (ft)	#438	256	#353	57	8	#259	48
Internal Link Dist (ft)		1966	2271		176		2689
Turn Bay Length (ft)	425			300			
Base Capacity (vph)	439	2148	1025	649	434	439	513
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.97	0.51	0.83	0.41	0.02	0.66	0.31

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
 2: Chambers Rd & 96th Ave

Year 2044 Background PM
 Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	415	1065	0	0	810	255	0	2	2	265	2	145
Future Volume (veh/h)	415	1065	0	0	810	255	0	2	2	265	2	145
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	428	1098	0	0	853	268	0	4	4	288	2	158
Peak Hour Factor	0.97	0.97	0.97	0.95	0.95	0.95	0.50	0.50	0.50	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	439	2099	0	0	981	437	0	9	9	345	4	304
Arrive On Green	0.25	0.59	0.00	0.00	0.28	0.28	0.00	0.01	0.01	0.19	0.19	0.19
Sat Flow, veh/h	1781	3647	0	0	3647	1585	0	858	858	1781	20	1568
Grp Volume(v), veh/h	428	1098	0	0	853	268	0	0	8	288	0	160
Grp Sat Flow(s),veh/h/ln	1781	1777	0	0	1777	1585	0	0	1716	1781	0	1588
Q Serve(g_s), s	17.4	13.4	0.0	0.0	16.7	10.8	0.0	0.0	0.3	11.4	0.0	6.6
Cycle Q Clear(g_c), s	17.4	13.4	0.0	0.0	16.7	10.8	0.0	0.0	0.3	11.4	0.0	6.6
Prop In Lane	1.00		0.00	0.00		1.00	0.00		0.50	1.00		0.99
Lane Grp Cap(c), veh/h	439	2099	0	0	981	437	0	0	18	345	0	308
V/C Ratio(X)	0.98	0.52	0.00	0.00	0.87	0.61	0.00	0.00	0.45	0.83	0.00	0.52
Avail Cap(c_a), veh/h	439	2140	0	0	1021	456	0	0	423	439	0	391
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	27.3	8.9	0.0	0.0	25.2	23.1	0.0	0.0	36.0	28.3	0.0	26.4
Incr Delay (d2), s/veh	36.4	0.2	0.0	0.0	8.0	2.3	0.0	0.0	17.2	10.6	0.0	1.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	11.4	4.4	0.0	0.0	7.7	4.1	0.0	0.0	0.2	5.5	0.0	2.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	63.7	9.1	0.0	0.0	33.2	25.3	0.0	0.0	53.2	38.9	0.0	27.8
LnGrp LOS	E	A	A	A	C	C	A	A	D	D	A	C
Approach Vol, veh/h		1526			1121			8			448	
Approach Delay, s/veh		24.4			31.3			53.2			35.0	
Approach LOS		C			C			D			C	
Timer - Assigned Phs		2		4	5	6		8				
Phs Duration (G+Y+Rc), s		48.2		19.2	23.0	25.2		5.7				
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		5.0				
Max Green Setting (Gmax), s		44.0		18.0	18.0	21.0		18.0				
Max Q Clear Time (g_c+I1), s		15.4		13.4	19.4	18.7		2.3				
Green Ext Time (p_c), s		9.5		0.8	0.0	1.5		0.0				
Intersection Summary												
HCM 6th Ctrl Delay					28.5							
HCM 6th LOS					C							

Timings
3: Chambers Rd & 100th Ave



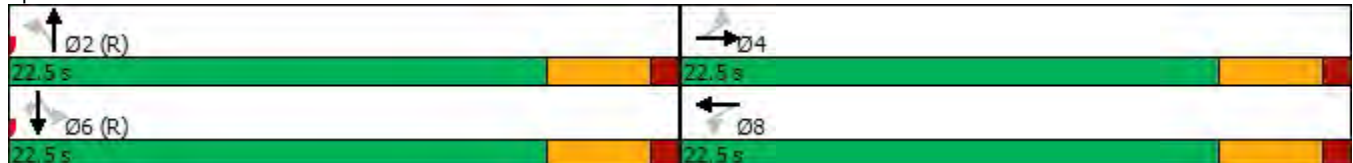
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Configurations	↶	↷	↶	↷	↶	↷	↶	↷	↶
Traffic Volume (vph)	55	8	45	11	55	565	245	450	85
Future Volume (vph)	55	8	45	11	55	565	245	450	85
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases		4		8		2		6	
Permitted Phases	4		8		2		6		6
Detector Phase	4	4	8	8	2	2	6	6	6
Switch Phase									
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (%)	50.0%	50.0%	50.0%	50.0%	50.0%	50.0%	50.0%	50.0%	50.0%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	8.4	8.4	8.4	8.4	30.5	30.5	30.5	30.5	30.5
Actuated g/C Ratio	0.19	0.19	0.19	0.19	0.68	0.68	0.68	0.68	0.68
v/c Ratio	0.30	0.09	0.19	0.51	0.10	0.58	0.70	0.39	0.08
Control Delay	18.3	8.5	15.6	10.4	5.4	9.7	24.6	6.4	1.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.3	8.5	15.6	10.4	5.4	9.7	24.6	6.4	1.9
LOS	B	A	B	B	A	A	C	A	A
Approach Delay		15.0		11.4		9.3		11.6	
Approach LOS		B		B		A		B	

Intersection Summary

Cycle Length: 45
 Actuated Cycle Length: 45
 Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.70
 Intersection Signal Delay: 10.8
 Intersection Capacity Utilization 80.6%
 Analysis Period (min) 15

Intersection LOS: B
 ICU Level of Service D

Splits and Phases: 3: Chambers Rd & 100th Ave



Queues
3: Chambers Rd & 100th Ave

Year 2044 Background PM
Chambers Rd Apartments Traffic Impact Study



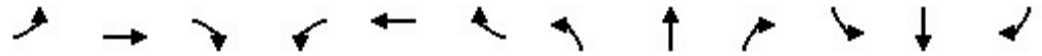
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	60	31	49	213	60	723	266	489	92
v/c Ratio	0.30	0.09	0.19	0.51	0.10	0.58	0.70	0.39	0.08
Control Delay	18.3	8.5	15.6	10.4	5.4	9.7	24.6	6.4	1.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.3	8.5	15.6	10.4	5.4	9.7	24.6	6.4	1.9
Queue Length 50th (ft)	14	2	11	16	5	84	37	49	0
Queue Length 95th (ft)	33	15	27	50	22	#307	#178	137	15
Internal Link Dist (ft)		1101		1411		2689		1855	
Turn Bay Length (ft)	150		100		235		235		135
Base Capacity (vph)	430	679	549	726	577	1239	381	1261	1101
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.14	0.05	0.09	0.29	0.10	0.58	0.70	0.39	0.08

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
 3: Chambers Rd & 100th Ave

Year 2044 Background PM
 Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	55	8	20	45	11	185	55	565	100	245	450	85
Future Volume (veh/h)	55	8	20	45	11	185	55	565	100	245	450	85
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	60	9	22	49	12	201	60	614	109	266	489	92
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	270	104	253	436	19	325	525	904	161	387	1094	927
Arrive On Green	0.22	0.22	0.22	0.22	0.22	0.22	0.58	0.58	0.58	0.58	0.58	0.58
Sat Flow, veh/h	1168	482	1177	1378	90	1509	833	1546	275	730	1870	1585
Grp Volume(v), veh/h	60	0	31	49	0	213	60	0	723	266	489	92
Grp Sat Flow(s),veh/h/ln	1168	0	1659	1378	0	1599	833	0	1821	730	1870	1585
Q Serve(g_s), s	2.2	0.0	0.7	1.3	0.0	5.4	2.0	0.0	12.3	14.0	6.6	1.2
Cycle Q Clear(g_c), s	7.6	0.0	0.7	2.0	0.0	5.4	8.6	0.0	12.3	26.3	6.6	1.2
Prop In Lane	1.00		0.71	1.00		0.94	1.00		0.15	1.00		1.00
Lane Grp Cap(c), veh/h	270	0	357	436	0	344	525	0	1065	387	1094	927
V/C Ratio(X)	0.22	0.00	0.09	0.11	0.00	0.62	0.11	0.00	0.68	0.69	0.45	0.10
Avail Cap(c_a), veh/h	486	0	663	691	0	640	525	0	1065	387	1094	927
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	0.00	1.00	0.58	0.00	0.58	1.00	1.00	1.00
Uniform Delay (d), s/veh	19.4	0.0	14.1	14.9	0.0	16.0	7.7	0.0	6.4	16.5	5.3	4.1
Incr Delay (d2), s/veh	0.4	0.0	0.1	0.1	0.0	1.8	0.3	0.0	2.0	9.5	1.3	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	0.0	0.2	0.4	0.0	1.9	0.3	0.0	2.8	3.2	1.6	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	19.9	0.0	14.2	15.0	0.0	17.8	7.9	0.0	8.5	26.0	6.6	4.3
LnGrp LOS	B	A	B	B	A	B	A	A	A	C	A	A
Approach Vol, veh/h		91			262			783				847
Approach Delay, s/veh		17.9			17.3			8.4				12.4
Approach LOS		B			B			A				B
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		30.8		14.2		30.8		14.2				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		18.0		18.0		18.0		18.0				
Max Q Clear Time (g_c+I1), s		14.3		9.6		28.3		7.4				
Green Ext Time (p_c), s		1.7		0.2		0.0		1.0				
Intersection Summary												
HCM 6th Ctrl Delay				11.8								
HCM 6th LOS				B								

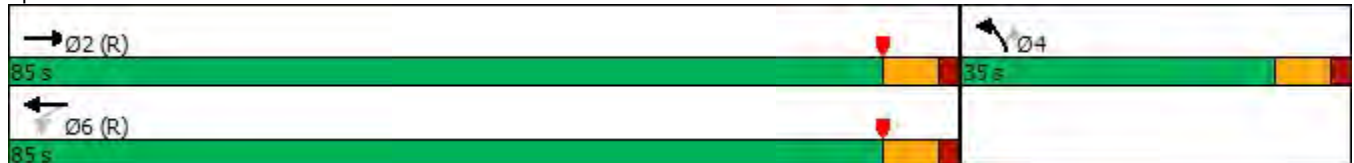
Timings
4: Sable Blvd & E. 104th Ave

	→	↙	←	↘	↗
Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Configurations	↑↑↑	↙	↑↑↑	↘	↗
Traffic Volume (vph)	2170	135	1870	110	85
Future Volume (vph)	2170	135	1870	110	85
Turn Type	NA	Perm	NA	Prot	Perm
Protected Phases	2		6	4	
Permitted Phases		6			4
Detector Phase	2	6	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	37.0	25.0	25.0	35.0	35.0
Total Split (s)	85.0	85.0	85.0	35.0	35.0
Total Split (%)	70.8%	70.8%	70.8%	29.2%	29.2%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	C-Min	C-Max	C-Max	None	None
Act Effct Green (s)	92.5	92.5	92.5	13.5	13.5
Actuated g/C Ratio	0.77	0.77	0.77	0.11	0.11
v/c Ratio	0.63	2.33	0.52	0.61	0.50
Control Delay	7.4	666.4	12.9	62.9	54.8
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	7.4	666.4	12.9	62.9	54.8
LOS	A	F	B	E	D
Approach Delay	7.4		57.0	59.4	
Approach LOS	A		E	E	

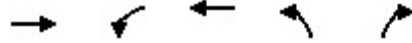
Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Yellow
 Natural Cycle: 150
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 2.33
 Intersection Signal Delay: 31.9
 Intersection Capacity Utilization 75.3%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service D

Splits and Phases: 4: Sable Blvd & E. 104th Ave



Queues
4: Sable Blvd & E. 104th Ave



Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	2473	147	2033	120	92
v/c Ratio	0.63	2.33	0.52	0.61	0.50
Control Delay	7.4	666.4	12.9	62.9	54.8
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	7.4	666.4	12.9	62.9	54.8
Queue Length 50th (ft)	263	~178	437	90	63
Queue Length 95th (ft)	369	m#282	557	147	112
Internal Link Dist (ft)	529		3380	1798	
Turn Bay Length (ft)		300		125	
Base Capacity (vph)	3897	63	3921	413	374
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.63	2.33	0.52	0.29	0.25

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
4: Sable Blvd & E. 104th Ave

Year 2044 Background PM
Chambers Rd Apartments Traffic Impact Study



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑↑		↵	↑↑↑	↵	↵
Traffic Volume (veh/h)	2170	105	135	1870	110	85
Future Volume (veh/h)	2170	105	135	1870	110	85
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	2359	114	147	2033	120	92
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	3974	191	142	4065	155	138
Arrive On Green	0.80	0.80	0.80	0.80	0.09	0.09
Sat Flow, veh/h	5160	239	135	5274	1781	1585
Grp Volume(v), veh/h	1603	870	147	2033	120	92
Grp Sat Flow(s),veh/h/ln	1702	1827	135	1702	1781	1585
Q Serve(g_s), s	21.8	22.2	73.3	16.2	7.9	6.7
Cycle Q Clear(g_c), s	21.8	22.2	95.5	16.2	7.9	6.7
Prop In Lane		0.13	1.00		1.00	1.00
Lane Grp Cap(c), veh/h	2710	1455	142	4065	155	138
V/C Ratio(X)	0.59	0.60	1.03	0.50	0.77	0.67
Avail Cap(c_a), veh/h	2710	1455	142	4065	416	370
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	0.68	0.68	1.00	1.00
Uniform Delay (d), s/veh	4.7	4.8	38.8	4.1	53.6	53.1
Incr Delay (d2), s/veh	1.0	1.8	71.0	0.3	7.9	5.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.3	6.1	7.0	3.7	3.9	2.9
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	5.7	6.6	109.8	4.4	61.5	58.5
LnGrp LOS	A	A	F	A	E	E
Approach Vol, veh/h	2473			2180	212	
Approach Delay, s/veh	6.0			11.5	60.2	
Approach LOS	A			B	E	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		102.5		17.5		102.5
Change Period (Y+Rc), s		7.0		7.0		7.0
Max Green Setting (Gmax), s		78.0		28.0		78.0
Max Q Clear Time (g_c+I1), s		24.2		9.9		97.5
Green Ext Time (p_c), s		32.8		0.6		0.0
Intersection Summary						
HCM 6th Ctrl Delay			10.8			
HCM 6th LOS			B			

Intersection												
Int Delay, s/veh	5.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	0	0	0	0	40	0	20	0	60	30	0
Future Vol, veh/h	0	0	0	0	0	40	0	20	0	60	30	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	0	43	0	22	0	65	33	0

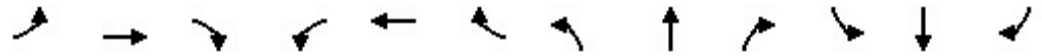
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	207	185	33	185	185	22	33	0	0	22	0	0
Stage 1	163	163	-	22	22	-	-	-	-	-	-	-
Stage 2	44	22	-	163	163	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	751	709	1041	776	709	1055	1579	-	-	1593	-	-
Stage 1	839	763	-	996	877	-	-	-	-	-	-	-
Stage 2	970	877	-	839	763	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	697	679	1041	751	679	1055	1579	-	-	1593	-	-
Mov Cap-2 Maneuver	697	679	-	751	679	-	-	-	-	-	-	-
Stage 1	839	731	-	996	877	-	-	-	-	-	-	-
Stage 2	930	877	-	804	731	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	0		8.6		0		4.9	
HCM LOS	A		A					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1579	-	-	-	-	1055	1593	-
HCM Lane V/C Ratio	-	-	-	-	-	0.041	0.041	-
HCM Control Delay (s)	0	-	-	0	8.6	7.4	0	-
HCM Lane LOS	A	-	-	A	A	A	A	-
HCM 95th %tile Q(veh)	0	-	-	-	0.1	0.1	-	-

Queues
1: Chambers Rd & E. 104th Ave

Year 2044 w/Project AM
Chambers Rd Apartments Traffic Impact Study





































Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	174	1217	233	148	1609	321	440	303	193	375	251	342
v/c Ratio	0.57	0.56	0.29	0.52	0.75	0.38	0.60	0.52	0.47	0.67	0.61	0.64
Control Delay	71.9	19.9	1.7	59.5	33.5	4.3	37.6	51.8	15.1	39.0	56.9	26.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	71.9	19.9	1.7	59.5	33.5	4.3	37.6	51.8	15.1	39.0	56.9	26.3
Queue Length 50th (ft)	74	131	0	57	384	0	142	116	9	114	98	127
Queue Length 95th (ft)	112	161	19	92	498	61	176	m153	m80	141	139	223
Internal Link Dist (ft)		2580			955			548			948	
Turn Bay Length (ft)	275		300	380		350	290		250	285		250
Base Capacity (vph)	318	2171	809	291	2134	850	838	855	517	556	530	535
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.55	0.56	0.29	0.51	0.75	0.38	0.53	0.35	0.37	0.67	0.47	0.64

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
1: Chambers Rd & E. 104th Ave

Year 2044 w/Project AM
Chambers Rd Apartments Traffic Impact Study

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	  		 	  		 	 		 	 	
Traffic Volume (veh/h)	160	1120	214	136	1480	295	405	279	178	345	231	315
Future Volume (veh/h)	160	1120	214	136	1480	295	405	279	178	345	231	315
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	174	1217	233	148	1609	0	440	303	193	375	251	342
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	228	2224	691	204	2189		707	641	286	619	533	342
Arrive On Green	0.13	0.87	0.87	0.06	0.43	0.00	0.13	0.18	0.18	0.09	0.15	0.15
Sat Flow, veh/h	3456	5106	1585	3456	5106	1585	3456	3554	1585	3456	3554	1585
Grp Volume(v), veh/h	174	1217	233	148	1609	0	440	303	193	375	251	342
Grp Sat Flow(s),veh/h/ln	1728	1702	1585	1728	1702	1585	1728	1777	1585	1728	1777	1585
Q Serve(g_s), s	5.8	7.0	3.2	5.1	31.5	0.0	12.6	9.2	13.6	11.0	7.8	18.0
Cycle Q Clear(g_c), s	5.8	7.0	3.2	5.1	31.5	0.0	12.6	9.2	13.6	11.0	7.8	18.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	228	2224	691	204	2189		707	641	286	619	533	342
V/C Ratio(X)	0.76	0.55	0.34	0.73	0.74		0.62	0.47	0.68	0.61	0.47	1.00
Avail Cap(c_a), veh/h	288	2224	691	259	2189		919	859	383	619	533	342
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.84	0.84	0.84	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	51.2	4.8	4.6	55.5	28.6	0.0	35.9	44.1	45.9	38.9	46.6	47.0
Incr Delay (d2), s/veh	7.6	0.8	1.1	7.2	2.2	0.0	0.9	0.5	2.8	1.7	0.6	48.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.6	1.7	1.1	2.4	12.6	0.0	5.3	4.0	5.5	4.8	3.4	14.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	58.8	5.6	5.7	62.8	30.8	0.0	36.9	44.6	48.7	40.6	47.3	95.4
LnGrp LOS	E	A	A	E	C		D	D	D	D	D	F
Approach Vol, veh/h		1624			1757			936			968	
Approach Delay, s/veh		11.3			33.5			41.8			61.7	
Approach LOS		B			C			D			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	14.1	59.3	18.0	28.6	14.9	58.4	21.6	25.0				
Change Period (Y+Rc), s	7.0	7.0	7.0	7.0	7.0	7.0	6.0	7.0				
Max Green Setting (Gmax), s	9.0	43.0	11.0	29.0	10.0	42.0	23.0	18.0				
Max Q Clear Time (g_c+I1), s	7.1	9.0	13.0	15.6	7.8	33.5	14.6	20.0				
Green Ext Time (p_c), s	0.1	11.0	0.0	2.0	0.1	5.9	1.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			33.3									
HCM 6th LOS			C									
Notes												
User approved pedestrian interval to be less than phase max green.												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Timings
2: Chambers Rd & 96th Ave

Year 2044 w/Project AM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	115	410	895	218	0	279	3
Future Volume (vph)	115	410	895	218	0	279	3
Turn Type	Prot	NA	NA	Perm	NA	Split	NA
Protected Phases	5	2	6		8	4	4
Permitted Phases				6			
Detector Phase	5	2	6	6	8	4	4
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Total Split (s)	23.0	49.0	26.0	26.0	23.0	23.0	23.0
Total Split (%)	24.2%	51.6%	27.4%	27.4%	24.2%	24.2%	24.2%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?	Yes		Yes	Yes			
Recall Mode	None	Min	None	None	None	None	None
Act Effct Green (s)	10.0	33.6	21.8	21.8	5.7	17.1	17.1
Actuated g/C Ratio	0.16	0.54	0.35	0.35	0.09	0.27	0.27
v/c Ratio	0.45	0.24	0.79	0.34	0.03	0.63	0.51
Control Delay	31.7	8.5	27.9	4.9	0.2	29.9	6.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	31.7	8.5	27.9	4.9	0.2	29.9	6.2
LOS	C	A	C	A	A	C	A
Approach Delay		13.5	23.4		0.2		17.2
Approach LOS		B	C		A		B

Intersection Summary

Cycle Length: 95
 Actuated Cycle Length: 62.7
 Natural Cycle: 95
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 0.79
 Intersection Signal Delay: 19.3
 Intersection Capacity Utilization 70.8%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service C

Splits and Phases: 2: Chambers Rd & 96th Ave



Queues
2: Chambers Rd & 96th Ave

Year 2044 w/Project AM
Chambers Rd Apartments Traffic Impact Study



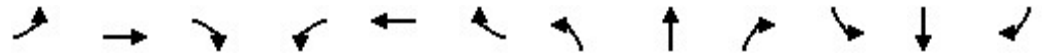
Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	125	448	973	237	10	303	348
v/c Ratio	0.45	0.24	0.79	0.34	0.03	0.63	0.51
Control Delay	31.7	8.5	27.9	4.9	0.2	29.9	6.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	31.7	8.5	27.9	4.9	0.2	29.9	6.2
Queue Length 50th (ft)	45	41	180	0	0	101	1
Queue Length 95th (ft)	104	88	#394	51	0	#255	66
Internal Link Dist (ft)		1966	2271		176		2584
Turn Bay Length (ft)	425			300		175	
Base Capacity (vph)	526	2571	1228	704	659	526	713
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.24	0.17	0.79	0.34	0.02	0.58	0.49

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
 2: Chambers Rd & 96th Ave

Year 2044 w/Project AM
 Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	115	410	2	0	895	218	0	0	5	279	3	317
Future Volume (veh/h)	115	410	2	0	895	218	0	0	5	279	3	317
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	125	446	2	0	973	237	0	0	10	303	3	345
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.50	0.50	0.50	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	183	1804	8	0	1124	501	0	0	20	453	3	400
Arrive On Green	0.10	0.50	0.50	0.00	0.32	0.32	0.00	0.00	0.01	0.25	0.25	0.25
Sat Flow, veh/h	1781	3628	16	0	3647	1585	0	0	1585	1781	14	1573
Grp Volume(v), veh/h	125	218	230	0	973	237	0	0	10	303	0	348
Grp Sat Flow(s),veh/h/ln	1781	1777	1867	0	1777	1585	0	0	1585	1781	0	1587
Q Serve(g_s), s	4.3	4.5	4.5	0.0	16.4	7.7	0.0	0.0	0.4	9.7	0.0	13.3
Cycle Q Clear(g_c), s	4.3	4.5	4.5	0.0	16.4	7.7	0.0	0.0	0.4	9.7	0.0	13.3
Prop In Lane	1.00		0.01	0.00		1.00	0.00		1.00	1.00		0.99
Lane Grp Cap(c), veh/h	183	884	929	0	1124	501	0	0	20	453	0	404
V/C Ratio(X)	0.68	0.25	0.25	0.00	0.87	0.47	0.00	0.00	0.50	0.67	0.00	0.86
Avail Cap(c_a), veh/h	503	1227	1289	0	1171	522	0	0	448	503	0	448
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	27.6	9.2	9.2	0.0	20.5	17.5	0.0	0.0	31.3	21.3	0.0	22.7
Incr Delay (d2), s/veh	4.5	0.1	0.1	0.0	6.8	0.7	0.0	0.0	17.6	3.0	0.0	14.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.0	1.5	1.6	0.0	7.2	2.7	0.0	0.0	0.2	4.0	0.0	6.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	32.1	9.3	9.3	0.0	27.3	18.2	0.0	0.0	48.8	24.3	0.0	37.3
LnGrp LOS	C	A	A	A	C	B	A	A	D	C	A	D
Approach Vol, veh/h		573			1210			10				651
Approach Delay, s/veh		14.3			25.5			48.8				31.2
Approach LOS		B			C			D				C
Timer - Assigned Phs		2		4	5	6		8				
Phs Duration (G+Y+Rc), s		36.7		21.2	11.5	25.2		5.8				
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		5.0				
Max Green Setting (Gmax), s		44.0		18.0	18.0	21.0		18.0				
Max Q Clear Time (g_c+I1), s		6.5		15.3	6.3	18.4		2.4				
Green Ext Time (p_c), s		2.9		0.9	0.2	1.7		0.0				
Intersection Summary												
HCM 6th Ctrl Delay				24.5								
HCM 6th LOS				C								

Timings
3: Chambers Rd & 100th Avenue

Year 2044 w/Project AM
Chambers Rd Apartments Traffic Impact Study

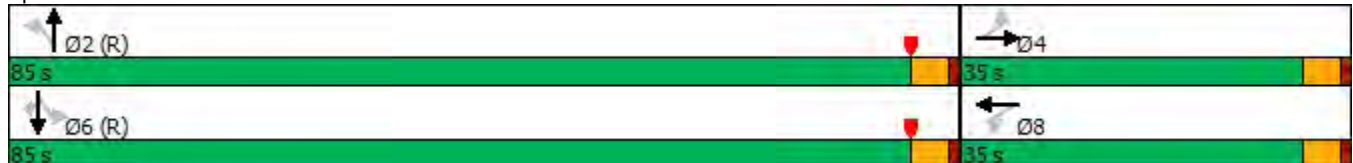


Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Configurations	↖	↗	↖	↗	↖	↗	↖	↗	↖
Traffic Volume (vph)	75	15	90	12	15	423	180	441	30
Future Volume (vph)	75	15	90	12	15	423	180	441	30
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases		4		8		2		6	
Permitted Phases	4		8		2		6		6
Detector Phase	4	4	8	8	2	2	6	6	6
Switch Phase									
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	35.0	35.0	35.0	35.0	85.0	85.0	85.0	85.0	85.0
Total Split (%)	29.2%	29.2%	29.2%	29.2%	70.8%	70.8%	70.8%	70.8%	70.8%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	17.5	17.5	17.5	17.5	93.5	93.5	93.5	93.5	93.5
Actuated g/C Ratio	0.15	0.15	0.15	0.15	0.78	0.78	0.78	0.78	0.78
v/c Ratio	1.32	0.21	0.50	0.56	0.02	0.36	0.31	0.33	0.03
Control Delay	264.1	18.4	54.1	11.8	4.6	5.6	5.0	4.4	0.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	264.1	18.4	54.1	11.8	4.6	5.6	5.0	4.4	0.8
LOS	F	B	D	B	A	A	A	A	A
Approach Delay		161.3		24.2		5.6		4.4	
Approach LOS		F		C		A		A	

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow
 Natural Cycle: 55
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.32
 Intersection Signal Delay: 21.5
 Intersection Capacity Utilization 67.7%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service C

Splits and Phases: 3: Chambers Rd & 100th Avenue



Queues
3: Chambers Rd & 100th Avenue

Year 2044 w/Project AM
Chambers Rd Apartments Traffic Impact Study



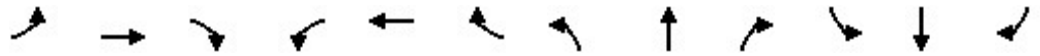
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	82	59	98	236	16	514	196	479	33
v/c Ratio	1.32	0.21	0.50	0.56	0.02	0.36	0.31	0.33	0.03
Control Delay	264.1	18.4	54.1	11.8	4.6	5.6	5.0	4.4	0.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	264.1	18.4	54.1	11.8	4.6	5.6	5.0	4.4	0.8
Queue Length 50th (ft)	~82	11	71	9	2	100	30	75	0
Queue Length 95th (ft)	#161	46	116	76	10	203	57	116	m4
Internal Link Dist (ft)		625		478		2584		1270	
Turn Bay Length (ft)	150		100		235		235		135
Base Capacity (vph)	108	453	340	572	671	1430	642	1451	1240
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.76	0.13	0.29	0.41	0.02	0.36	0.31	0.33	0.03

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
 3: Chambers Rd & 100th Avenue

Year 2044 w/Project AM
 Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	75	15	40	90	12	205	15	423	50	180	441	30
Future Volume (veh/h)	75	15	40	90	12	205	15	423	50	180	441	30
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	82	16	43	98	13	223	16	460	54	196	479	33
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	161	100	269	321	20	337	592	1153	135	580	1313	1113
Arrive On Green	0.22	0.22	0.22	0.22	0.22	0.22	0.70	0.70	0.70	0.70	0.70	0.70
Sat Flow, veh/h	1144	448	1205	1344	88	1510	888	1643	193	887	1870	1585
Grp Volume(v), veh/h	82	0	59	98	0	236	16	0	514	196	479	33
Grp Sat Flow(s),veh/h/ln	1144	0	1653	1344	0	1598	888	0	1836	887	1870	1585
Q Serve(g_s), s	8.4	0.0	3.4	7.6	0.0	16.1	0.9	0.0	13.9	14.1	12.3	0.8
Cycle Q Clear(g_c), s	24.6	0.0	3.4	11.1	0.0	16.1	13.2	0.0	13.9	28.0	12.3	0.8
Prop In Lane	1.00		0.73	1.00		0.94	1.00		0.11	1.00		1.00
Lane Grp Cap(c), veh/h	161	0	369	321	0	357	592	0	1288	580	1313	1113
V/C Ratio(X)	0.51	0.00	0.16	0.31	0.00	0.66	0.03	0.00	0.40	0.34	0.36	0.03
Avail Cap(c_a), veh/h	197	0	420	363	0	406	592	0	1288	580	1313	1113
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	0.00	1.00	0.94	0.00	0.94	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.7	0.0	37.6	42.0	0.0	42.5	9.8	0.0	7.4	13.2	7.2	5.4
Incr Delay (d2), s/veh	2.5	0.0	0.2	0.5	0.0	3.3	0.1	0.0	0.9	1.6	0.8	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.5	0.0	1.4	2.6	0.0	6.7	0.2	0.0	5.0	2.9	4.5	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	56.2	0.0	37.8	42.5	0.0	45.8	9.9	0.0	8.3	14.8	8.0	5.5
LnGrp LOS	E	A	D	D	A	D	A	A	A	B	A	A
Approach Vol, veh/h		141			334			530			708	
Approach Delay, s/veh		48.5			44.8			8.3			9.7	
Approach LOS		D			D			A			A	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		88.7		31.3		88.7		31.3				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		80.5		30.5		80.5		30.5				
Max Q Clear Time (g_c+I1), s		15.9		26.6		30.0		18.1				
Green Ext Time (p_c), s		3.6		0.2		4.6		1.4				
Intersection Summary												
HCM 6th Ctrl Delay				19.3								
HCM 6th LOS				B								

Timings
4: Sable Blvd & E. 104th Ave

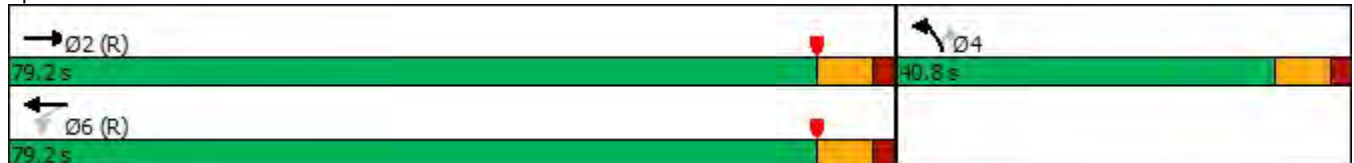
	→	↙	←	↘	↗
Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Configurations	↑↑↑	↙	↑↑↑	↘	↗
Traffic Volume (vph)	1669	65	2045	238	110
Future Volume (vph)	1669	65	2045	238	110
Turn Type	NA	Perm	NA	Prot	Perm
Protected Phases	2		6	4	
Permitted Phases		6			4
Detector Phase	2	6	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	37.0	22.5	22.5	40.5	40.5
Total Split (s)	79.2	79.2	79.2	40.8	40.8
Total Split (%)	66.0%	66.0%	66.0%	34.0%	34.0%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	C-Max	C-Max	C-Max	None	None
Act Effct Green (s)	83.1	83.1	83.1	22.9	22.9
Actuated g/C Ratio	0.69	0.69	0.69	0.19	0.19
v/c Ratio	0.53	0.61	0.63	0.77	0.38
Control Delay	10.2	49.3	23.5	60.8	37.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	10.2	49.3	23.5	60.8	37.9
LOS	B	D	C	E	D
Approach Delay	10.2		24.3	53.6	
Approach LOS	B		C	D	

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 117.5 (98%), Referenced to phase 2:EBT and 6:WBTL, Start of Yellow
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.77
 Intersection Signal Delay: 21.0
 Intersection Capacity Utilization 67.9%
 Analysis Period (min) 15

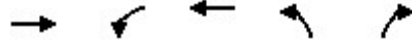
Intersection LOS: C
 ICU Level of Service C

Splits and Phases: 4: Sable Blvd & E. 104th Ave



Queues
4: Sable Blvd & E. 104th Ave

Year 2044 w/Project AM
Chambers Rd Apartments Traffic Impact Study









Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1855	71	2223	259	120
v/c Ratio	0.53	0.61	0.63	0.77	0.38
Control Delay	10.2	49.3	23.5	60.8	37.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	10.2	49.3	23.5	60.8	37.9
Queue Length 50th (ft)	229	44	505	192	69
Queue Length 95th (ft)	326	m86	674	267	118
Internal Link Dist (ft)	374		2580	1227	
Turn Bay Length (ft)		300		125	
Base Capacity (vph)	3514	116	3523	498	458
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.53	0.61	0.63	0.52	0.26

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
4: Sable Blvd & E. 104th Ave

Year 2044 w/Project AM
Chambers Rd Apartments Traffic Impact Study

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑↑		↵	↑↑↑	↵	↵
Traffic Volume (veh/h)	1669	38	65	2045	238	110
Future Volume (veh/h)	1669	38	65	2045	238	110
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	1814	41	71	2223	259	120
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	3680	83	199	3658	297	265
Arrive On Green	0.72	0.72	0.72	0.72	0.17	0.17
Sat Flow, veh/h	5306	116	248	5274	1781	1585
Grp Volume(v), veh/h	1202	653	71	2223	259	120
Grp Sat Flow(s),veh/h/ln	1702	1849	248	1702	1781	1585
Q Serve(g_s), s	18.6	18.6	21.1	26.2	17.0	8.2
Cycle Q Clear(g_c), s	18.6	18.6	39.7	26.2	17.0	8.2
Prop In Lane		0.06	1.00		1.00	1.00
Lane Grp Cap(c), veh/h	2438	1325	199	3658	297	265
V/C Ratio(X)	0.49	0.49	0.36	0.61	0.87	0.45
Avail Cap(c_a), veh/h	2438	1325	199	3658	502	446
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	0.64	0.64	1.00	1.00
Uniform Delay (d), s/veh	7.5	7.5	16.2	8.6	48.7	45.0
Incr Delay (d2), s/veh	0.7	1.3	3.2	0.5	8.6	1.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.7	6.5	1.3	7.9	8.2	3.3
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	8.2	8.8	19.3	9.0	57.3	46.3
LnGrp LOS	A	A	B	A	E	D
Approach Vol, veh/h	1855			2294	379	
Approach Delay, s/veh	8.4			9.4	53.8	
Approach LOS	A			A	D	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		93.0		27.0		93.0
Change Period (Y+Rc), s		7.0		7.0		7.0
Max Green Setting (Gmax), s		72.2		33.8		72.2
Max Q Clear Time (g_c+I1), s		20.6		19.0		41.7
Green Ext Time (p_c), s		19.4		1.0		22.7
Intersection Summary						
HCM 6th Ctrl Delay			12.7			
HCM 6th LOS			B			

Intersection												
Int Delay, s/veh	5.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	0	0	0	0	63	0	25	0	23	10	0
Future Vol, veh/h	0	0	0	0	0	63	0	25	0	23	10	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	0	68	0	27	0	25	11	0

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	122	88	11	88	88	27	11	0	0	27	0	0
Stage 1	61	61	-	27	27	-	-	-	-	-	-	-
Stage 2	61	27	-	61	61	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	853	802	1070	897	802	1048	1608	-	-	1587	-	-
Stage 1	950	844	-	990	873	-	-	-	-	-	-	-
Stage 2	950	873	-	950	844	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	787	789	1070	886	789	1048	1608	-	-	1587	-	-
Mov Cap-2 Maneuver	787	789	-	886	789	-	-	-	-	-	-	-
Stage 1	950	830	-	990	873	-	-	-	-	-	-	-
Stage 2	888	873	-	935	830	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	0		8.7		0		5.1	
HCM LOS	A		A					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1608	-	-	-	-	1048	1587	-
HCM Lane V/C Ratio	-	-	-	-	-	0.065	0.016	-
HCM Control Delay (s)	0	-	-	0	8.7	7.3	0	-
HCM Lane LOS	A	-	-	A	A	A	A	-
HCM 95th %tile Q(veh)	0	-	-	-	0.2	0	-	-

Intersection						
Int Delay, s/veh	2.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↖	↗	↖	↗	↗	↖
Traffic Vol, veh/h	72	26	8	680	595	26
Future Vol, veh/h	72	26	8	680	595	26
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	50	175	-	-	135
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	78	28	9	739	647	28

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1404	647	675	0	-	0
Stage 1	647	-	-	-	-	-
Stage 2	757	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	154	471	916	-	-	-
Stage 1	521	-	-	-	-	-
Stage 2	463	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	152	471	916	-	-	-
Mov Cap-2 Maneuver	152	-	-	-	-	-
Stage 1	516	-	-	-	-	-
Stage 2	463	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	41.2	0.1	0
HCM LOS	E		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	916	-	152	471	-	-
HCM Lane V/C Ratio	0.009	-	0.515	0.06	-	-
HCM Control Delay (s)	9	-	51.4	13.1	-	-
HCM Lane LOS	A	-	F	B	-	-
HCM 95th %tile Q(veh)	0	-	2.5	0.2	-	-

Intersection						
Int Delay, s/veh	3.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Vol, veh/h	1	46	26	8	52	4
Future Vol, veh/h	1	46	26	8	52	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	50	28	9	57	4

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	37	0	-	0	85 33
Stage 1	-	-	-	-	33 -
Stage 2	-	-	-	-	52 -
Critical Hdwy	4.12	-	-	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	2.218	-	-	-	3.518 3.318
Pot Cap-1 Maneuver	1574	-	-	-	916 1041
Stage 1	-	-	-	-	989 -
Stage 2	-	-	-	-	970 -
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	1574	-	-	-	915 1041
Mov Cap-2 Maneuver	-	-	-	-	915 -
Stage 1	-	-	-	-	988 -
Stage 2	-	-	-	-	970 -

Approach	EB	WB	SB
HCM Control Delay, s	0.2	0	9.2
HCM LOS			A

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1574	-	-	-	923
HCM Lane V/C Ratio	0.001	-	-	-	0.066
HCM Control Delay (s)	7.3	0	-	-	9.2
HCM Lane LOS	A	A	-	-	A
HCM 95th %tile Q(veh)	0	-	-	-	0.2

Intersection												
Int Delay, s/veh	5.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	1	5	5	5	5	8	5	0	5	26	0	4
Future Vol, veh/h	1	5	5	5	5	8	5	0	5	26	0	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	5	5	5	5	9	5	0	5	28	0	4

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	14	0	0	10	0	0	32	34	8	32	32	10
Stage 1	-	-	-	-	-	-	10	10	-	20	20	-
Stage 2	-	-	-	-	-	-	22	24	-	12	12	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1604	-	-	1610	-	-	976	859	1074	976	861	1071
Stage 1	-	-	-	-	-	-	1011	887	-	999	879	-
Stage 2	-	-	-	-	-	-	996	875	-	1009	886	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1604	-	-	1610	-	-	969	856	1074	968	858	1071
Mov Cap-2 Maneuver	-	-	-	-	-	-	969	856	-	968	858	-
Stage 1	-	-	-	-	-	-	1010	886	-	998	876	-
Stage 2	-	-	-	-	-	-	989	872	-	1003	885	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.7	2	8.6	8.8
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	1019	1604	-	-	1610	-	-	981
HCM Lane V/C Ratio	0.011	0.001	-	-	0.003	-	-	0.033
HCM Control Delay (s)	8.6	7.2	0	-	7.2	0	-	8.8
HCM Lane LOS	A	A	A	-	A	A	-	A
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.1

Timings
1: Chambers Rd & E. 104th Ave

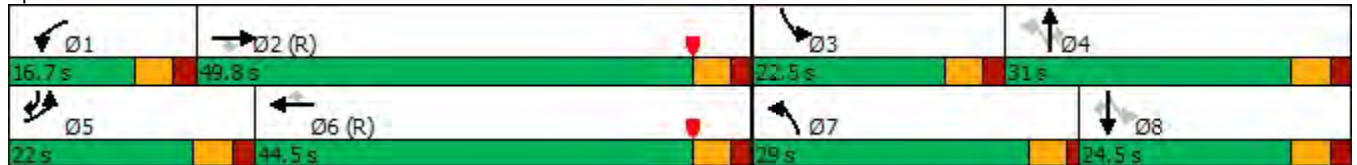
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	340	1240	317	163	1245	395	379	428	180	475	435	145
Future Volume (vph)	340	1240	317	163	1245	395	379	428	180	475	435	145
Turn Type	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	pm+ov
Protected Phases	5	2		1	6		7	4		3	8	5
Permitted Phases			2			6	4		4	8		8
Detector Phase	5	2	2	1	6	6	7	4	4	3	8	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.5	23.5	23.5	10.5	24.5	24.5	9.5	23.5	23.5	10.5	24.5	10.5
Total Split (s)	22.0	49.8	49.8	16.7	44.5	44.5	29.0	31.0	31.0	22.5	24.5	22.0
Total Split (%)	18.3%	41.5%	41.5%	13.9%	37.1%	37.1%	24.2%	25.8%	25.8%	18.8%	20.4%	18.3%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.5	5.5	5.5	5.5	5.5	5.5	4.5	5.5	5.5	5.5	5.5	5.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	C-Min	C-Min	None	C-Min	C-Min	None	None	None	None	None	None
Act Effct Green (s)	16.5	49.2	49.2	10.7	43.4	43.4	38.3	21.5	21.5	39.9	23.3	45.3
Actuated g/C Ratio	0.14	0.41	0.41	0.09	0.36	0.36	0.32	0.18	0.18	0.33	0.19	0.38
v/c Ratio	0.78	0.65	0.40	0.58	0.74	0.52	0.63	0.73	0.44	0.80	0.69	0.24
Control Delay	74.3	24.4	2.0	60.4	37.1	6.1	32.8	55.6	12.4	39.0	50.6	8.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	74.3	24.4	2.0	60.4	37.1	6.1	32.8	55.6	12.4	39.0	50.6	8.8
LOS	E	C	A	E	D	A	C	E	B	D	D	A
Approach Delay		29.6			32.4			39.0			39.6	
Approach LOS		C			C			D			D	

Intersection Summary

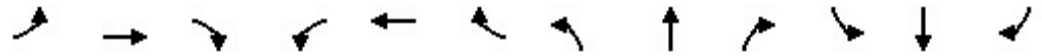
Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Yellow
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.80
 Intersection Signal Delay: 33.9
 Intersection LOS: C
 Intersection Capacity Utilization 77.5%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 1: Chambers Rd & E. 104th Ave



Queues
1: Chambers Rd & E. 104th Ave

Year 2044 w/Project PM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	370	1348	345	177	1353	429	412	465	196	516	473	158
v/c Ratio	0.78	0.65	0.40	0.58	0.74	0.52	0.63	0.73	0.44	0.80	0.69	0.24
Control Delay	74.3	24.4	2.0	60.4	37.1	6.1	32.8	55.6	12.4	39.0	50.6	8.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	74.3	24.4	2.0	60.4	37.1	6.1	32.8	55.6	12.4	39.0	50.6	8.8
Queue Length 50th (ft)	144	280	0	68	342	11	122	186	14	151	178	21
Queue Length 95th (ft)	#211	287	25	106	412	91	m157	m234	m69	189	239	67
Internal Link Dist (ft)		2580			955			548			948	
Turn Bay Length (ft)	275		300	380		350	290		250	285		250
Base Capacity (vph)	487	2086	853	327	1839	832	880	752	487	654	687	674
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.76	0.65	0.40	0.54	0.74	0.52	0.47	0.62	0.40	0.79	0.69	0.23

Intersection Summary



































95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
 1: Chambers Rd & E. 104th Ave

Year 2044 w/Project PM
 Chambers Rd Apartments Traffic Impact Study

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	  		 	  		 	 		 	 	
Traffic Volume (veh/h)	340	1240	317	163	1245	395	379	428	180	475	435	145
Future Volume (veh/h)	340	1240	317	163	1245	395	379	428	180	475	435	145
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	370	1348	345	177	1353	0	412	465	196	516	473	158
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	422	2282	708	235	2005		641	576	257	657	666	491
Arrive On Green	0.24	0.89	0.89	0.07	0.39	0.00	0.12	0.16	0.16	0.14	0.19	0.19
Sat Flow, veh/h	3456	5106	1585	3456	5106	1585	3456	3554	1585	3456	3554	1585
Grp Volume(v), veh/h	370	1348	345	177	1353	0	412	465	196	516	473	158
Grp Sat Flow(s),veh/h/ln	1728	1702	1585	1728	1702	1585	1728	1777	1585	1728	1777	1585
Q Serve(g_s), s	12.4	7.1	4.9	6.0	26.3	0.0	11.6	15.1	14.2	14.7	15.0	9.2
Cycle Q Clear(g_c), s	12.4	7.1	4.9	6.0	26.3	0.0	11.6	15.1	14.2	14.7	15.0	9.2
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	422	2282	708	235	2005		641	576	257	657	666	491
V/C Ratio(X)	0.88	0.59	0.49	0.75	0.67		0.64	0.81	0.76	0.79	0.71	0.32
Avail Cap(c_a), veh/h	475	2282	708	323	2005		922	755	337	663	666	491
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.72	0.72	0.72	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	44.5	3.9	3.8	54.9	30.1	0.0	35.7	48.5	48.1	35.7	45.7	31.8
Incr Delay (d2), s/veh	11.8	0.8	1.7	6.4	1.8	0.0	1.1	4.9	7.3	6.1	3.5	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.2	1.5	1.4	2.8	10.6	0.0	4.9	7.0	6.0	6.6	6.8	3.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	56.2	4.7	5.5	61.4	31.9	0.0	36.7	53.4	55.4	41.9	49.2	32.1
LnGrp LOS	E	A	A	E	C		D	D	E	D	D	C
Approach Vol, veh/h		2063			1530			1073			1147	
Approach Delay, s/veh		14.1			35.3			47.4			43.6	
Approach LOS		B			D			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	13.7	59.1	22.3	24.9	20.2	52.6	19.2	28.0				
Change Period (Y+Rc), s	5.5	5.5	5.5	5.5	5.5	5.5	4.5	5.5				
Max Green Setting (Gmax), s	11.2	44.3	17.0	25.5	16.5	39.0	24.5	19.0				
Max Q Clear Time (g_c+I1), s	8.0	9.1	16.7	17.1	14.4	28.3	13.6	17.0				
Green Ext Time (p_c), s	0.1	13.4	0.1	2.3	0.3	6.2	1.1	0.7				

Intersection Summary

HCM 6th Ctrl Delay	31.6
HCM 6th LOS	C

Notes

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

Timings
2: Chambers Rd & 96th Ave

Year 2044 w/Project PM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	433	1065	810	264	2	270	2
Future Volume (vph)	433	1065	810	264	2	270	2
Turn Type	Prot	NA	NA	Perm	NA	Split	NA
Protected Phases	5	2	6		8	4	4
Permitted Phases				6			
Detector Phase	5	2	6	6	8	4	4
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Total Split (s)	23.0	49.0	26.0	26.0	23.0	23.0	23.0
Total Split (%)	24.2%	51.6%	27.4%	27.4%	24.2%	24.2%	24.2%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?	Yes		Yes	Yes			
Recall Mode	None	Min	None	None	None	None	None
Act Effect Green (s)	18.1	44.2	21.1	21.1	5.9	16.7	16.7
Actuated g/C Ratio	0.25	0.61	0.29	0.29	0.08	0.23	0.23
v/c Ratio	1.02	0.51	0.83	0.42	0.06	0.73	0.35
Control Delay	79.0	10.2	34.4	5.6	27.5	38.8	6.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	79.0	10.2	34.4	5.6	27.5	38.8	6.9
LOS	E	B	C	A	C	D	A
Approach Delay		30.0	27.3		27.5		27.0
Approach LOS		C	C		C		C

Intersection Summary

Cycle Length: 95
 Actuated Cycle Length: 73
 Natural Cycle: 105
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 1.02
 Intersection Signal Delay: 28.6
 Intersection Capacity Utilization 86.0%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service E

Splits and Phases: 2: Chambers Rd & 96th Ave



Queues
2: Chambers Rd & 96th Ave

Year 2044 w/Project PM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBT	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	446	1098	853	278	8	293	172
v/c Ratio	1.02	0.51	0.83	0.42	0.06	0.73	0.35
Control Delay	79.0	10.2	34.4	5.6	27.5	38.8	6.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	79.0	10.2	34.4	5.6	27.5	38.8	6.9
Queue Length 50th (ft)	~201	128	184	0	2	117	1
Queue Length 95th (ft)	#462	256	#353	58	8	#266	50
Internal Link Dist (ft)		1966	2271		176		2584
Turn Bay Length (ft)	425			300		175	
Base Capacity (vph)	438	2143	1023	655	433	438	521
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	1.02	0.51	0.83	0.42	0.02	0.67	0.33

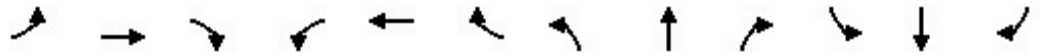
Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary
2: Chambers Rd & 96th Ave

Year 2044 w/Project PM
Chambers Rd Apartments Traffic Impact Study



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	433	1065	0	0	810	264	0	2	2	270	2	156
Future Volume (veh/h)	433	1065	0	0	810	264	0	2	2	270	2	156
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	446	1098	0	0	853	278	0	4	4	293	2	170
Peak Hour Factor	0.97	0.97	0.97	0.95	0.95	0.95	0.50	0.50	0.50	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	437	2093	0	0	979	436	0	9	9	350	4	308
Arrive On Green	0.25	0.59	0.00	0.00	0.28	0.28	0.00	0.01	0.01	0.20	0.20	0.20
Sat Flow, veh/h	1781	3647	0	0	3647	1585	0	858	858	1781	18	1569
Grp Volume(v), veh/h	446	1098	0	0	853	278	0	0	8	293	0	172
Grp Sat Flow(s),veh/h/ln	1781	1777	0	0	1777	1585	0	0	1716	1781	0	1588
Q Serve(g_s), s	18.0	13.5	0.0	0.0	16.8	11.3	0.0	0.0	0.3	11.6	0.0	7.2
Cycle Q Clear(g_c), s	18.0	13.5	0.0	0.0	16.8	11.3	0.0	0.0	0.3	11.6	0.0	7.2
Prop In Lane	1.00		0.00	0.00		1.00	0.00		0.50	1.00		0.99
Lane Grp Cap(c), veh/h	437	2093	0	0	979	436	0	0	18	350	0	312
V/C Ratio(X)	1.02	0.52	0.00	0.00	0.87	0.64	0.00	0.00	0.45	0.84	0.00	0.55
Avail Cap(c_a), veh/h	437	2131	0	0	1017	454	0	0	421	437	0	390
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	27.7	9.0	0.0	0.0	25.3	23.4	0.0	0.0	36.1	28.3	0.0	26.6
Incr Delay (d2), s/veh	48.4	0.2	0.0	0.0	8.2	2.8	0.0	0.0	17.2	11.1	0.0	1.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	13.1	4.4	0.0	0.0	7.8	4.3	0.0	0.0	0.2	5.7	0.0	2.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	76.0	9.2	0.0	0.0	33.5	26.2	0.0	0.0	53.3	39.5	0.0	28.1
LnGrp LOS	F	A	A	A	C	C	A	A	D	D	A	C
Approach Vol, veh/h		1544			1131			8				465
Approach Delay, s/veh		28.5			31.7			53.3				35.3
Approach LOS		C			C			D				D
Timer - Assigned Phs		2		4	5	6		8				
Phs Duration (G+Y+Rc), s		48.2		19.4	23.0	25.2		5.8				
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		5.0				
Max Green Setting (Gmax), s		44.0		18.0	18.0	21.0		18.0				
Max Q Clear Time (g_c+I1), s		15.5		13.6	20.0	18.8		2.3				
Green Ext Time (p_c), s		9.5		0.8	0.0	1.4		0.0				
Intersection Summary												
HCM 6th Ctrl Delay					30.7							
HCM 6th LOS					C							

Timings
3: Chambers Rd & 100th Avenue

Year 2044 w/Project PM
Chambers Rd Apartments Traffic Impact Study

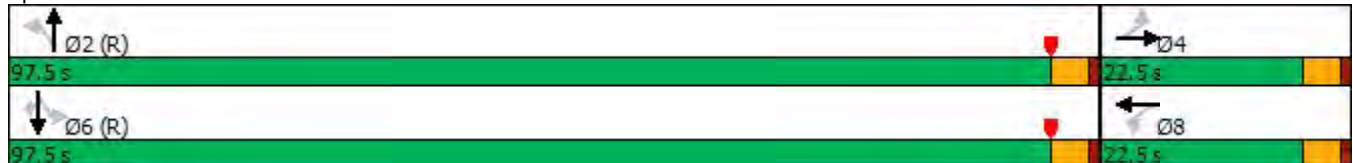


Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Configurations	↖	↗	↖	↗	↖	↗	↖	↗	↖
Traffic Volume (vph)	55	8	45	11	55	592	245	466	85
Future Volume (vph)	55	8	45	11	55	592	245	466	85
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases		4		8		2		6	
Permitted Phases	4		8		2		6		6
Detector Phase	4	4	8	8	2	2	6	6	6
Switch Phase									
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	22.5	22.5	22.5	22.5	97.5	97.5	97.5	97.5	97.5
Total Split (%)	18.8%	18.8%	18.8%	18.8%	81.3%	81.3%	81.3%	81.3%	81.3%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	15.5	15.5	15.5	15.5	95.5	95.5	95.5	95.5	95.5
Actuated g/C Ratio	0.13	0.13	0.13	0.13	0.80	0.80	0.80	0.80	0.80
v/c Ratio	0.97	0.13	0.28	0.56	0.09	0.52	0.55	0.34	0.07
Control Delay	157.5	23.5	49.9	13.6	3.5	5.9	11.8	2.7	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	157.5	23.5	49.9	13.6	3.5	5.9	11.8	2.7	0.2
LOS	F	C	D	B	A	A	B	A	A
Approach Delay		111.9		20.4		5.8		5.2	
Approach LOS		F		C		A		A	

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.97
 Intersection Signal Delay: 12.2
 Intersection Capacity Utilization 82.0%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service D

Splits and Phases: 3: Chambers Rd & 100th Avenue



Queues
3: Chambers Rd & 100th Avenue

Year 2044 w/Project PM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	60	31	49	213	60	752	266	507	92
v/c Ratio	0.97	0.13	0.28	0.56	0.09	0.52	0.55	0.34	0.07
Control Delay	157.5	23.5	49.9	13.6	3.5	5.9	11.8	2.7	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	157.5	23.5	49.9	13.6	3.5	5.9	11.8	2.7	0.2
Queue Length 50th (ft)	46	6	34	8	9	179	40	61	0
Queue Length 95th (ft)	#130	35	73	80	20	251	125	73	m1
Internal Link Dist (ft)		625		478		2584		1270	
Turn Bay Length (ft)	150		100		235		235		135
Base Capacity (vph)	72	268	205	410	669	1454	486	1482	1278
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.12	0.24	0.52	0.09	0.52	0.55	0.34	0.07

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
3: Chambers Rd & 100th Avenue

Year 2044 w/Project PM
Chambers Rd Apartments Traffic Impact Study

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	55	8	20	45	11	185	55	592	100	245	466	85
Future Volume (veh/h)	55	8	20	45	11	185	55	592	100	245	466	85
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	60	9	22	49	12	201	60	643	109	266	507	92
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	83	72	177	244	14	226	677	1208	205	499	1450	1228
Arrive On Green	0.15	0.15	0.15	0.15	0.15	0.15	0.77	0.77	0.77	0.77	0.77	0.77
Sat Flow, veh/h	1168	482	1177	1378	90	1509	892	1559	264	711	1870	1585
Grp Volume(v), veh/h	60	0	31	49	0	213	60	0	752	266	507	92
Grp Sat Flow(s),veh/h/ln	1168	0	1659	1378	0	1599	892	0	1823	711	1870	1585
Q Serve(g_s), s	2.3	0.0	1.9	3.8	0.0	15.7	2.7	0.0	19.0	27.5	10.0	1.7
Cycle Q Clear(g_c), s	18.0	0.0	1.9	5.8	0.0	15.7	12.7	0.0	19.0	46.4	10.0	1.7
Prop In Lane	1.00		0.71	1.00		0.94	1.00		0.14	1.00		1.00
Lane Grp Cap(c), veh/h	83	0	249	244	0	240	677	0	1413	499	1450	1228
V/C Ratio(X)	0.73	0.00	0.12	0.20	0.00	0.89	0.09	0.00	0.53	0.53	0.35	0.07
Avail Cap(c_a), veh/h	83	0	249	244	0	240	677	0	1413	499	1450	1228
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	0.00	1.00	0.53	0.00	0.53	1.00	1.00	1.00
Uniform Delay (d), s/veh	59.6	0.0	44.2	46.7	0.0	50.0	6.1	0.0	5.2	14.1	4.2	3.2
Incr Delay (d2), s/veh	27.1	0.0	0.2	0.4	0.0	30.6	0.1	0.0	0.8	4.1	0.7	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.4	0.0	0.8	1.3	0.0	8.3	0.5	0.0	6.2	4.8	3.5	0.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	86.6	0.0	44.4	47.1	0.0	80.6	6.3	0.0	5.9	18.1	4.8	3.3
LnGrp LOS	F	A	D	D	A	F	A	A	A	B	A	A
Approach Vol, veh/h		91			262			812			865	
Approach Delay, s/veh		72.2			74.3			6.0			8.8	
Approach LOS		E			E			A			A	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		97.5		22.5		97.5		22.5				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		93.0		18.0		93.0		18.0				
Max Q Clear Time (g_c+I1), s		21.0		20.0		48.4		17.7				
Green Ext Time (p_c), s		7.5		0.0		7.1		0.0				
Intersection Summary												
HCM 6th Ctrl Delay				18.9								
HCM 6th LOS				B								

Timings
4: Sable Blvd & E. 104th Ave

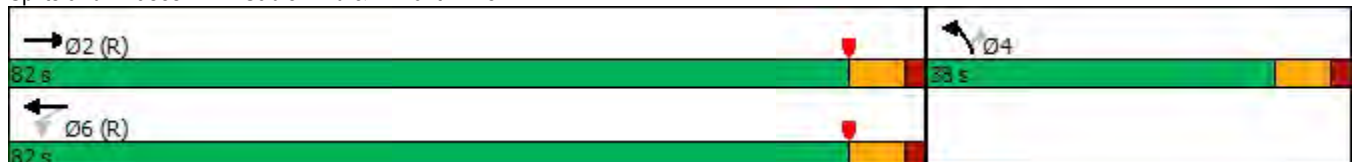
	→	↙	←	↘	↗
Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Configurations	↑↑↑	↙	↑↑↑	↘	↗
Traffic Volume (vph)	2202	135	1889	115	85
Future Volume (vph)	2202	135	1889	115	85
Turn Type	NA	Perm	NA	Prot	Perm
Protected Phases	2		6	4	
Permitted Phases		6			4
Detector Phase	2	6	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	37.0	22.5	22.5	35.0	35.0
Total Split (s)	82.0	82.0	82.0	38.0	38.0
Total Split (%)	68.3%	68.3%	68.3%	31.7%	31.7%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	C-Max	C-Max	C-Max	None	None
Act Effct Green (s)	92.2	92.2	92.2	13.8	13.8
Actuated g/C Ratio	0.77	0.77	0.77	0.12	0.12
v/c Ratio	0.65	2.41	0.53	0.62	0.49
Control Delay	7.8	693.4	9.1	62.9	55.1
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	7.8	693.4	9.1	62.9	55.1
LOS	A	F	A	E	E
Approach Delay	7.8		54.9	59.6	
Approach LOS	A		D	E	

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Yellow
 Natural Cycle: 150
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 2.41
 Intersection Signal Delay: 31.1
 Intersection Capacity Utilization 76.4%
 Analysis Period (min) 15

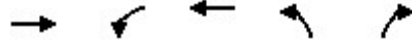
Intersection LOS: C
 ICU Level of Service D

Splits and Phases: 4: Sable Blvd & E. 104th Ave



Queues
4: Sable Blvd & E. 104th Ave

Year 2044 w/Project PM
Chambers Rd Apartments Traffic Impact Study



Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	2517	147	2053	125	92
v/c Ratio	0.65	2.41	0.53	0.62	0.49
Control Delay	7.8	693.4	9.1	62.9	55.1
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	7.8	693.4	9.1	62.9	55.1
Queue Length 50th (ft)	276	~177	416	94	64
Queue Length 95th (ft)	389	m#289	476	151	113
Internal Link Dist (ft)	374		2580	1227	
Turn Bay Length (ft)		300		125	
Base Capacity (vph)	3882	61	3905	457	412
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.65	2.41	0.53	0.27	0.22

Intersection Summary







~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary
4: Sable Blvd & E. 104th Ave

Year 2044 w/Project PM
Chambers Rd Apartments Traffic Impact Study

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑↑		↙	↑↑↑	↘	↗
Traffic Volume (veh/h)	2202	114	135	1889	115	85
Future Volume (veh/h)	2202	114	135	1889	115	85
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	2393	124	147	2053	125	92
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	3944	203	137	4049	161	143
Arrive On Green	0.79	0.79	0.53	0.53	0.09	0.09
Sat Flow, veh/h	5141	255	129	5274	1781	1585
Grp Volume(v), veh/h	1631	886	147	2053	125	92
Grp Sat Flow(s),veh/h/ln	1702	1824	129	1702	1781	1585
Q Serve(g_s), s	22.9	23.4	71.7	30.9	8.2	6.7
Cycle Q Clear(g_c), s	22.9	23.4	95.2	30.9	8.2	6.7
Prop In Lane		0.14	1.00		1.00	1.00
Lane Grp Cap(c), veh/h	2700	1447	137	4049	161	143
V/C Ratio(X)	0.60	0.61	1.07	0.51	0.78	0.64
Avail Cap(c_a), veh/h	2700	1447	137	4049	460	409
HCM Platoon Ratio	1.00	1.00	0.67	0.67	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.69	0.69	1.00	1.00
Uniform Delay (d), s/veh	4.9	5.0	55.5	13.1	53.4	52.7
Incr Delay (d2), s/veh	1.0	1.9	84.2	0.3	7.8	4.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.7	7.7	7.5	13.0	4.0	2.9
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	5.9	6.9	139.7	13.4	61.2	57.5
LnGrp LOS	A	A	F	B	E	E
Approach Vol, veh/h	2517			2200	217	
Approach Delay, s/veh	6.3			21.8	59.6	
Approach LOS	A			C	E	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		102.2		17.8		102.2
Change Period (Y+Rc), s		7.0		7.0		7.0
Max Green Setting (Gmax), s		75.0		31.0		75.0
Max Q Clear Time (g_c+I1), s		25.4		10.2		97.2
Green Ext Time (p_c), s		35.6		0.6		0.0
Intersection Summary						
HCM 6th Ctrl Delay			15.6			
HCM 6th LOS			B			

Intersection												
Int Delay, s/veh	5.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	0	0	0	0	45	0	20	0	69	30	0
Future Vol, veh/h	0	0	0	0	0	45	0	20	0	69	30	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	0	49	0	22	0	75	33	0

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	230	205	33	205	205	22	33	0	0	22	0	0
Stage 1	183	183	-	22	22	-	-	-	-	-	-	-
Stage 2	47	22	-	183	183	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	725	691	1041	753	691	1055	1579	-	-	1593	-	-
Stage 1	819	748	-	996	877	-	-	-	-	-	-	-
Stage 2	967	877	-	819	748	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	666	658	1041	725	658	1055	1579	-	-	1593	-	-
Mov Cap-2 Maneuver	666	658	-	725	658	-	-	-	-	-	-	-
Stage 1	819	712	-	996	877	-	-	-	-	-	-	-
Stage 2	922	877	-	780	712	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	0		8.6		0		5.1	
HCM LOS	A		A					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1579	-	-	-	1055	1593	-	-
HCM Lane V/C Ratio	-	-	-	-	0.046	0.047	-	-
HCM Control Delay (s)	0	-	-	0	8.6	7.4	0	-
HCM Lane LOS	A	-	-	A	A	A	A	-
HCM 95th %tile Q(veh)	0	-	-	-	0.1	0.1	-	-

Intersection						
Int Delay, s/veh	2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	42	16	27	765	720	75
Future Vol, veh/h	42	16	27	765	720	75
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	50	175	-	-	135
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	46	17	29	832	783	82

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1673	783	865	0	-	0
Stage 1	783	-	-	-	-	-
Stage 2	890	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	105	394	778	-	-	-
Stage 1	450	-	-	-	-	-
Stage 2	401	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	101	394	778	-	-	-
Mov Cap-2 Maneuver	101	-	-	-	-	-
Stage 1	433	-	-	-	-	-
Stage 2	401	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	52.7	0.3	0
HCM LOS	F		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	778	-	101	394	-	-
HCM Lane V/C Ratio	0.038	-	0.452	0.044	-	-
HCM Control Delay (s)	9.8	-	67.2	14.6	-	-
HCM Lane LOS	A	-	F	B	-	-
HCM 95th %tile Q(veh)	0.1	-	1.9	0.1	-	-

Intersection						
Int Delay, s/veh	2.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Vol, veh/h	5	26	75	27	32	3
Future Vol, veh/h	5	26	75	27	32	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	5	28	82	29	35	3

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	111	0	-	0	135 97
Stage 1	-	-	-	-	97 -
Stage 2	-	-	-	-	38 -
Critical Hdwy	4.12	-	-	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	2.218	-	-	-	3.518 3.318
Pot Cap-1 Maneuver	1479	-	-	-	859 959
Stage 1	-	-	-	-	927 -
Stage 2	-	-	-	-	984 -
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	1479	-	-	-	856 959
Mov Cap-2 Maneuver	-	-	-	-	856 -
Stage 1	-	-	-	-	924 -
Stage 2	-	-	-	-	984 -

Approach	EB	WB	SB
HCM Control Delay, s	1.2	0	9.4
HCM LOS			A

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1479	-	-	-	864
HCM Lane V/C Ratio	0.004	-	-	-	0.044
HCM Control Delay (s)	7.4	0	-	-	9.4
HCM Lane LOS	A	A	-	-	A
HCM 95th %tile Q(veh)	0	-	-	-	0.1

Intersection												
Int Delay, s/veh	4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	5	5	5	5	5	27	5	0	5	16	0	3
Future Vol, veh/h	5	5	5	5	5	27	5	0	5	16	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	5	5	5	5	5	29	5	0	5	17	0	3

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	34	0	0	10	0	0	49	62	8	50	50	20
Stage 1	-	-	-	-	-	-	18	18	-	30	30	-
Stage 2	-	-	-	-	-	-	31	44	-	20	20	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1578	-	-	1610	-	-	951	829	1074	950	841	1058
Stage 1	-	-	-	-	-	-	1001	880	-	987	870	-
Stage 2	-	-	-	-	-	-	986	858	-	999	879	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1578	-	-	1610	-	-	943	824	1074	941	836	1058
Mov Cap-2 Maneuver	-	-	-	-	-	-	943	824	-	941	836	-
Stage 1	-	-	-	-	-	-	998	877	-	984	867	-
Stage 2	-	-	-	-	-	-	980	855	-	991	876	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	2.4	1	8.6	8.8
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	1004	1578	-	-	1610	-	-	958
HCM Lane V/C Ratio	0.011	0.003	-	-	0.003	-	-	0.022
HCM Control Delay (s)	8.6	7.3	0	-	7.2	0	-	8.8
HCM Lane LOS	A	A	A	-	A	A	-	A
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.1

*MUTCD Peak Hour Signal Warrant
102nd Avenue & Chambers Road*



Figure 4C-3. Warrant 3, Peak Hour



*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Traffic Count Data Sheets

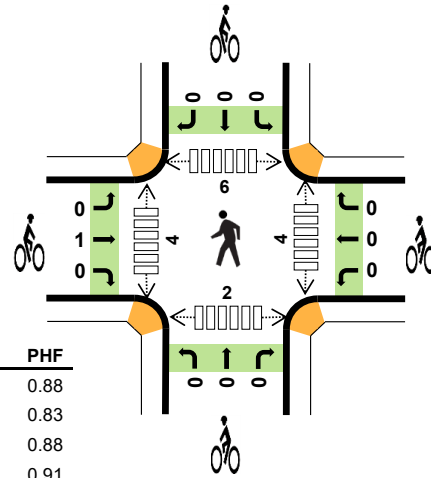
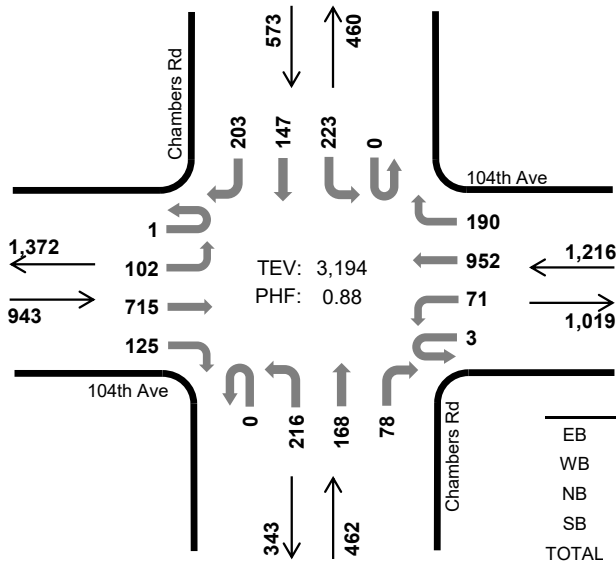


Chambers Rd 104th Ave



Peak Hour

Date: 11/14/2023
Count Period: 7:00 AM to 9:00 AM
Peak Hour: 7:30 AM to 8:30 AM



	HV %:	PHF
EB	5.3%	0.88
WB	2.8%	0.83
NB	3.2%	0.88
SB	2.1%	0.91
TOTAL	3.5%	0.88

Two-Hour Count Summaries

Interval Start	104th Ave Eastbound				104th Ave Westbound				Chambers Rd Northbound				Chambers Rd Southbound				15-min Total	Rolling One Hour	
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
7:00 AM	0	7	146	18	0	3	185	30	0	35	45	8	0	32	47	32	588	0	
7:15 AM	0	15	173	29	0	11	202	28	0	42	36	15	0	46	45	55	697	0	
7:30 AM	0	26	198	43	0	10	206	40	0	65	31	28	0	42	36	65	790	0	
7:45 AM	0	38	186	28	0	29	292	45	0	48	58	26	0	58	40	60	908	2,983	
8:00 AM	1	20	142	25	3	19	264	51	0	57	42	15	0	68	35	49	791	3,186	
8:15 AM	0	18	189	29	0	13	190	54	0	46	37	9	0	55	36	29	705	3,194	
8:30 AM	0	24	132	25	0	9	150	39	0	32	31	4	0	59	45	23	573	2,977	
8:45 AM	0	23	122	22	0	5	143	32	0	28	25	11	0	26	24	29	490	2,559	
Count Total	1	171	1,288	219	3	99	1,632	319	0	353	305	116	0	386	308	342	5,542	0	
Peak Hour	All	1	102	715	125	3	71	952	190	0	216	168	78	0	223	147	203	3,194	0
	HV	0	5	41	4	0	2	25	7	0	5	9	1	0	5	5	2	111	0
	HV%	0%	5%	6%	3%	0%	3%	3%	4%	-	2%	5%	1%	-	2%	3%	1%	3%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	16	5	3	2	26	0	0	0	0	0	1	0	2	0	3
7:15 AM	13	4	3	5	25	0	0	0	0	0	1	1	1	0	3
7:30 AM	16	9	3	3	31	1	0	0	0	1	3	0	3	1	7
7:45 AM	13	10	3	6	32	0	0	0	0	0	0	3	1	0	4
8:00 AM	7	6	7	1	21	0	0	0	0	0	1	0	2	0	3
8:15 AM	14	9	2	2	27	0	0	0	0	0	0	1	0	1	2
8:30 AM	7	6	2	1	16	0	0	0	0	0	0	0	0	1	1
8:45 AM	9	6	1	2	18	0	0	0	0	0	0	0	0	1	1
Count Total	95	55	24	22	196	1	0	0	0	1	6	5	9	4	24
Peak Hour	50	34	15	12	111	1	0	0	0	1	4	4	6	2	16

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	104th Ave				104th Ave				Chambers Rd				Chambers Rd				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
7:00 AM	0	0	16	0	0	0	5	0	0	0	2	1	0	0	2	0	26	0
7:15 AM	0	1	10	2	0	1	1	2	0	1	1	1	0	2	1	2	25	0
7:30 AM	0	1	13	2	0	0	8	1	0	1	1	1	0	2	1	0	31	0
7:45 AM	0	4	7	2	0	2	6	2	0	1	2	0	0	1	3	2	32	114
8:00 AM	0	0	7	0	0	0	6	0	0	2	5	0	0	1	0	0	21	109
8:15 AM	0	0	14	0	0	0	5	4	0	1	1	0	0	1	1	0	27	111
8:30 AM	0	1	5	1	0	0	4	2	0	1	1	0	0	0	1	0	16	96
8:45 AM	0	2	7	0	0	0	4	2	0	1	0	0	0	1	0	1	18	82
Count Total	0	9	79	7	0	3	39	13	0	8	13	3	0	8	9	5	196	0
Peak Hour	0	5	41	4	0	2	25	7	0	5	9	1	0	5	5	2	111	0

Two-Hour Count Summaries - Bikes																		
Interval Start	104th Ave			104th Ave			Chambers Rd			Chambers Rd			15-min Total	Rolling One Hour				
	Eastbound			Westbound			Northbound			Southbound								
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT						
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Count Total	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
Peak Hour	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0

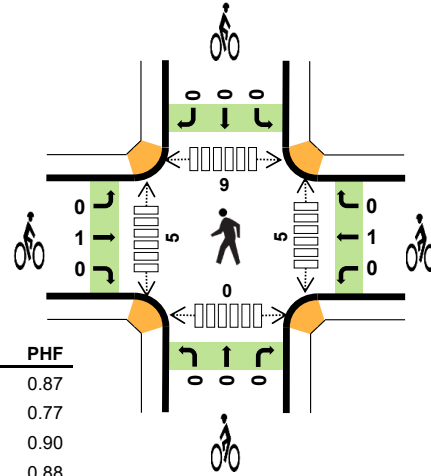
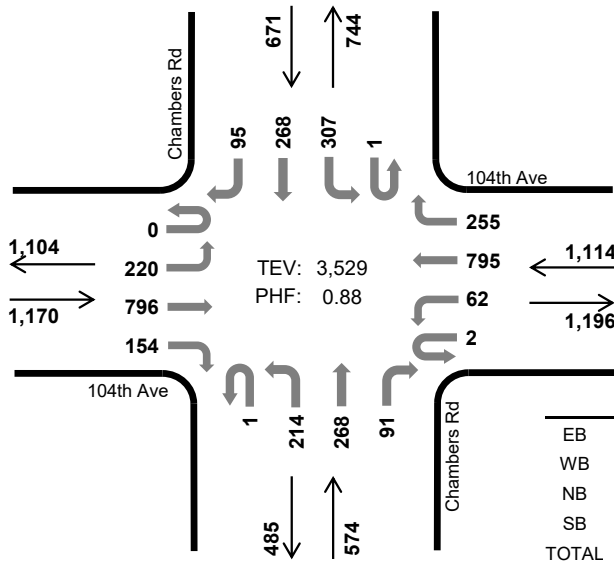
Note: U-Turn volumes for bikes are included in Left-Turn, if any.

Chambers Rd 104th Ave



Peak Hour

Date: 11/14/2023
Count Period: 4:00 PM to 6:00 PM
Peak Hour: 4:15 PM to 5:15 PM



	HV %:	PHF
EB	1.1%	0.87
WB	3.0%	0.77
NB	0.7%	0.90
SB	1.0%	0.88
TOTAL	1.6%	0.88

Two-Hour Count Summaries

Interval Start	104th Ave Eastbound				104th Ave Westbound				Chambers Rd Northbound				Chambers Rd Southbound				15-min Total	Rolling One Hour	
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	51	175	31	0	15	178	48	2	45	45	15	0	79	58	23	765	0	
4:15 PM	0	42	193	36	0	18	181	48	1	43	63	17	0	84	78	19	823	0	
4:30 PM	0	56	191	30	2	14	200	59	0	59	66	29	0	73	61	16	856	0	
4:45 PM	0	76	216	43	0	16	261	84	0	62	75	23	1	66	57	26	1,006	3,450	
5:00 PM	0	46	196	45	0	14	153	64	0	50	64	22	0	84	72	34	844	3,529	
5:15 PM	0	55	205	38	0	12	159	60	0	42	49	18	2	65	58	22	785	3,491	
5:30 PM	0	67	163	21	0	10	147	69	0	51	66	24	0	64	68	21	771	3,406	
5:45 PM	0	56	156	28	0	11	135	57	0	45	43	14	0	76	51	30	702	3,102	
Count Total	0	449	1,495	272	2	110	1,414	489	3	397	471	162	3	591	503	191	6,552	0	
Peak Hour	All	0	220	796	154	2	62	795	255	1	214	268	91	1	307	268	95	3,529	0
	HV	0	1	8	4	0	3	26	4	0	2	2	0	0	3	0	4	57	0
	HV%	-	0%	1%	3%	0%	5%	3%	2%	0%	1%	1%	0%	0%	1%	0%	4%	2%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	4	11	0	1	16	0	0	0	0	0	0	0	3	0	3
4:15 PM	5	8	0	1	14	0	0	0	0	0	2	0	0	0	2
4:30 PM	2	8	1	1	12	1	0	0	0	1	0	0	3	0	3
4:45 PM	1	10	1	3	15	0	1	0	0	1	3	3	4	0	10
5:00 PM	5	7	2	2	16	0	0	0	0	0	0	2	2	0	4
5:15 PM	5	9	5	3	22	0	0	0	0	0	2	0	3	0	5
5:30 PM	3	8	1	1	13	1	0	0	0	1	1	2	1	1	5
5:45 PM	3	12	1	1	17	0	0	0	0	0	0	0	0	0	0
Count Total	28	73	11	13	125	2	1	0	0	3	8	7	16	1	32
Peak Hour	13	33	4	7	57	1	1	0	0	2	5	5	9	0	19

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	104th Ave				104th Ave				Chambers Rd				Chambers Rd				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	1	1	2	0	0	11	0	0	0	0	0	0	1	0	0	16	0
4:15 PM	0	0	5	0	0	2	4	2	0	0	0	0	0	1	0	0	14	0
4:30 PM	0	0	0	2	0	1	7	0	0	1	0	0	0	0	0	1	12	0
4:45 PM	0	1	0	0	0	0	9	1	0	0	1	0	0	0	0	3	15	57
5:00 PM	0	0	3	2	0	0	6	1	0	1	1	0	0	2	0	0	16	57
5:15 PM	0	0	3	2	0	0	8	1	0	1	4	0	0	1	2	0	22	65
5:30 PM	0	1	2	0	0	0	8	0	0	1	0	0	0	0	0	1	13	66
5:45 PM	0	0	3	0	0	1	9	2	0	0	0	1	0	0	0	1	17	68
Count Total	0	3	17	8	0	4	62	7	0	4	6	1	0	5	2	6	125	0
Peak Hour	0	1	8	4	0	3	26	4	0	2	2	0	0	3	0	4	57	0

Two-Hour Count Summaries - Bikes																		
Interval Start	104th Ave			104th Ave			Chambers Rd			Chambers Rd			15-min Total	Rolling One Hour				
	Eastbound			Westbound			Northbound			Southbound								
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT						
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
4:45 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	2
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
5:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	2
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Count Total	0	2	0	0	0	1	0	0	0	0	0	0	0	0	0	0	3	0
Peak Hour	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	2	0

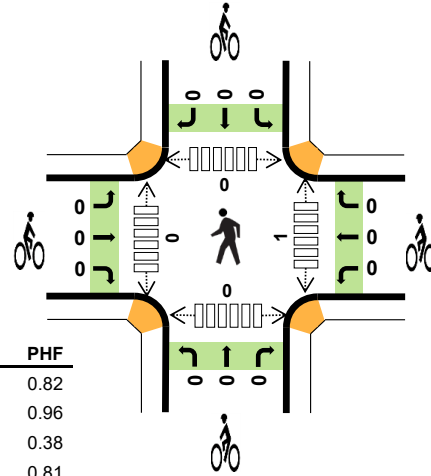
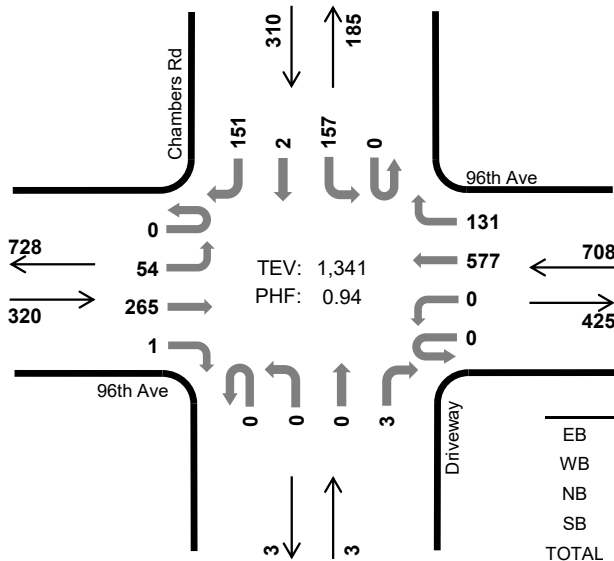
Note: U-Turn volumes for bikes are included in Left-Turn, if any.

Chambers Rd 96th Ave



Peak Hour

Date: 11/14/2023
Count Period: 7:00 AM to 9:00 AM
Peak Hour: 7:15 AM to 8:15 AM



	HV %:	PHF
EB	24.1%	0.82
WB	6.5%	0.96
NB	0.0%	0.38
SB	2.9%	0.81
TOTAL	9.8%	0.94

Two-Hour Count Summaries

Interval Start	96th Ave Eastbound				96th Ave Westbound				Driveway Northbound				Chambers Rd Southbound				15-min Total	Rolling One Hour	
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
7:00 AM	0	7	46	0	0	0	151	26	0	0	0	0	0	45	0	45	320	0	
7:15 AM	0	12	59	0	0	0	134	29	0	0	0	0	0	41	0	25	300	0	
7:30 AM	0	14	83	1	0	0	154	29	0	0	0	1	0	39	1	36	358	0	
7:45 AM	0	17	65	0	0	0	144	34	0	0	0	0	0	48	0	48	356	1,334	
8:00 AM	0	11	58	0	0	0	145	39	0	0	0	2	0	29	1	42	327	1,341	
8:15 AM	0	11	76	1	0	1	116	21	0	0	0	0	0	33	0	26	285	1,326	
8:30 AM	0	17	71	0	0	0	102	26	0	0	0	0	0	25	0	16	257	1,225	
8:45 AM	0	10	75	0	0	0	84	16	0	0	0	0	0	24	0	13	222	1,091	
Count Total	0	99	533	2	0	1	1,030	220	0	0	0	3	0	284	2	251	2,425	0	
Peak Hour	All	0	54	265	1	0	0	577	131	0	0	0	3	0	157	2	151	1,341	0
	HV	0	3	74	0	0	0	40	6	0	0	0	0	0	6	0	3	132	0
	HV%	-	6%	28%	0%	-	-	7%	5%	-	-	-	0%	-	4%	0%	2%	10%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	25	12	0	3	40	0	0	0	0	0	0	0	0	0	0
7:15 AM	11	6	0	1	18	0	0	0	0	0	0	0	0	0	0
7:30 AM	25	14	0	0	39	0	0	0	0	0	0	0	0	0	0
7:45 AM	23	13	0	4	40	0	0	0	0	0	1	0	0	0	1
8:00 AM	18	13	0	4	35	0	0	0	0	0	0	0	0	0	0
8:15 AM	13	19	0	1	33	0	0	0	0	0	0	0	0	0	0
8:30 AM	18	16	0	0	34	0	0	0	0	0	0	0	0	0	0
8:45 AM	20	11	0	1	32	0	0	0	0	0	0	0	0	0	0
Count Total	153	104	0	14	271	0	0	0	0	0	1	0	0	0	1
Peak Hour	77	46	0	9	132	0	0	0	0	0	1	0	0	0	1

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	96th Ave				96th Ave				Driveway				Chambers Rd				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
7:00 AM	0	1	24	0	0	0	11	1	0	0	0	0	0	2	0	1	40	0
7:15 AM	0	1	10	0	0	0	6	0	0	0	0	0	0	1	0	0	18	0
7:30 AM	0	1	24	0	0	0	12	2	0	0	0	0	0	0	0	0	39	0
7:45 AM	0	1	22	0	0	0	12	1	0	0	0	0	0	2	0	2	40	137
8:00 AM	0	0	18	0	0	0	10	3	0	0	0	0	0	3	0	1	35	132
8:15 AM	0	0	13	0	0	0	18	1	0	0	0	0	0	0	0	1	33	147
8:30 AM	0	1	17	0	0	0	16	0	0	0	0	0	0	0	0	0	34	142
8:45 AM	0	0	20	0	0	0	9	2	0	0	0	0	0	1	0	0	32	134
Count Total	0	5	148	0	0	0	94	10	0	0	0	0	0	9	0	5	271	0
Peak Hour	0	3	74	0	0	0	40	6	0	0	0	0	0	6	0	3	132	0

Two-Hour Count Summaries - Bikes																	
Interval Start	96th Ave			96th Ave			Driveway			Chambers Rd			15-min Total	Rolling One Hour			
	Eastbound			Westbound			Northbound			Southbound							
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT					
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Count Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Peak Hour	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

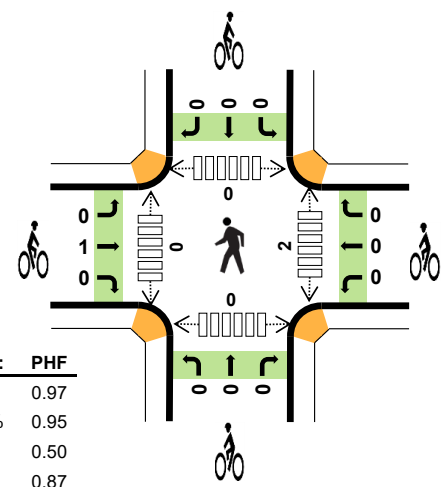
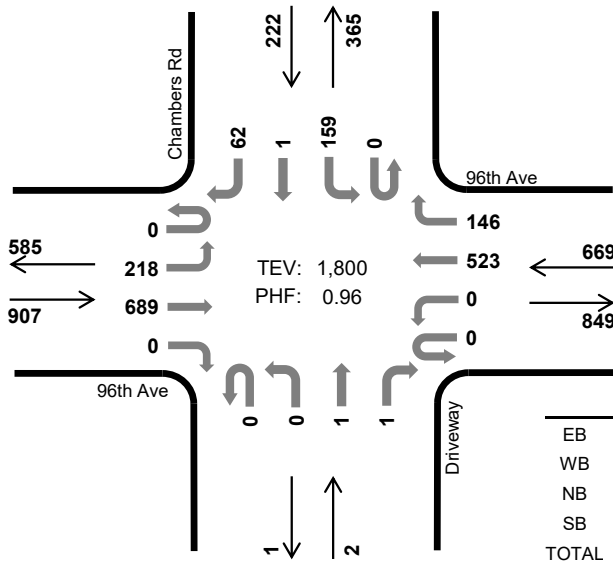
Note: U-Turn volumes for bikes are included in Left-Turn, if any.

Chambers Rd 96th Ave



Peak Hour

Date: 11/14/2023
Count Period: 4:00 PM to 6:00 PM
Peak Hour: 4:30 PM to 5:30 PM



	HV %:	PHF
EB	3.2%	0.97
WB	15.4%	0.95
NB	0.0%	0.50
SB	2.3%	0.87
TOTAL	7.6%	0.96

Two-Hour Count Summaries

Interval Start	96th Ave Eastbound				96th Ave Westbound				Driveway Northbound				Chambers Rd Southbound				15-min Total	Rolling One Hour	
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	52	128	0	0	0	101	33	0	0	0	0	0	31	0	10	355	0	
4:15 PM	0	54	149	0	0	0	121	34	0	0	0	0	0	24	0	18	400	0	
4:30 PM	0	59	158	0	0	0	138	38	0	0	1	0	0	34	0	13	441	0	
4:45 PM	0	59	173	0	0	0	130	44	0	0	0	0	0	47	0	17	470	1,666	
5:00 PM	0	49	185	0	0	0	138	31	0	0	0	0	0	39	0	15	457	1,768	
5:15 PM	0	51	173	0	0	0	117	33	0	0	0	1	0	39	1	17	432	1,800	
5:30 PM	0	46	160	0	0	0	105	42	0	0	0	0	0	30	0	12	395	1,754	
5:45 PM	0	41	158	1	0	0	95	48	0	0	0	1	0	29	0	17	390	1,674	
Count Total	0	411	1,284	1	0	0	945	303	0	0	1	2	0	273	1	119	3,340	0	
Peak Hour	All	0	218	689	0	0	0	523	146	0	0	1	1	0	159	1	62	1,800	0
	HV	0	1	28	0	0	0	102	1	0	0	0	0	0	4	0	1	137	0
	HV%	-	0%	4%	-	-	-	20%	1%	-	-	0%	0%	-	3%	0%	2%	8%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	8	26	0	0	34	0	0	0	0	0	0	0	0	0	0
4:15 PM	5	32	0	1	38	0	0	0	0	0	0	0	0	0	0
4:30 PM	9	31	0	2	42	0	0	0	0	0	1	0	0	0	1
4:45 PM	9	24	0	1	34	1	0	0	0	1	0	0	0	0	0
5:00 PM	7	33	0	1	41	0	0	0	0	0	1	0	0	0	1
5:15 PM	4	15	0	1	20	0	0	0	0	0	0	0	0	0	0
5:30 PM	4	19	0	2	25	0	0	0	0	0	0	0	0	0	0
5:45 PM	7	20	0	1	28	0	0	0	0	0	1	0	0	1	2
Count Total	53	200	0	9	262	1	0	0	0	1	3	0	0	1	4
Peak Hour	29	103	0	5	137	1	0	0	0	1	2	0	0	0	2

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	96th Ave				96th Ave				Driveway				Chambers Rd				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	0	8	0	0	0	24	2	0	0	0	0	0	0	0	0	34	0
4:15 PM	0	0	5	0	0	0	32	0	0	0	0	0	0	0	0	1	38	0
4:30 PM	0	1	8	0	0	0	31	0	0	0	0	0	0	2	0	0	42	0
4:45 PM	0	0	9	0	0	0	24	0	0	0	0	0	0	1	0	0	34	148
5:00 PM	0	0	7	0	0	0	32	1	0	0	0	0	0	1	0	0	41	155
5:15 PM	0	0	4	0	0	0	15	0	0	0	0	0	0	0	0	1	20	137
5:30 PM	0	0	4	0	0	0	18	1	0	0	0	0	0	1	0	1	25	120
5:45 PM	0	0	7	0	0	0	20	0	0	0	0	0	0	0	0	1	28	114
Count Total	0	1	52	0	0	0	196	4	0	0	0	0	0	5	0	4	262	0
Peak Hour	0	1	28	0	0	0	102	1	0	0	0	0	0	4	0	1	137	0

Two-Hour Count Summaries - Bikes																		
Interval Start	96th Ave			96th Ave			Driveway			Chambers Rd			15-min Total	Rolling One Hour				
	Eastbound			Westbound			Northbound			Southbound								
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT						
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Count Total	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
Peak Hour	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0

Note: U-Turn volumes for bikes are included in Left-Turn, if any.

Location: Chambers Rd N/O 101st Ave
 Date Range: 11/14/2023 - 11/20/2023
 Site Code: 01

Time	Tuesday 11/14/2023			Wednesday 11/15/2023			Thursday 11/16/2023			Friday 11/17/2023			Saturday 11/18/2023			Sunday 11/19/2023			Monday 11/20/2023			Mid-Week Average		
	NB	SB	Total	NB	SB	Total	NB	SB	Total	NB	SB	Total	NB	SB	Total	NB	SB	Total	NB	SB	Total	NB	SB	Total
12:00 AM	17	13	30	9	11	20	-	-	-	-	-	-	-	-	-	-	-	-	13	12	25			
1:00 AM	10	14	24	11	12	23	-	-	-	-	-	-	-	-	-	-	-	-	11	13	24			
2:00 AM	8	8	16	6	7	13	-	-	-	-	-	-	-	-	-	-	-	-	7	8	15			
3:00 AM	15	15	30	13	14	27	-	-	-	-	-	-	-	-	-	-	-	-	14	15	29			
4:00 AM	35	27	62	24	24	48	-	-	-	-	-	-	-	-	-	-	-	-	30	26	55			
5:00 AM	68	59	127	78	48	126	-	-	-	-	-	-	-	-	-	-	-	-	73	54	127			
6:00 AM	170	192	362	194	220	414	-	-	-	-	-	-	-	-	-	-	-	-	182	206	388			
7:00 AM	411	364	775	382	356	738	-	-	-	-	-	-	-	-	-	-	-	-	397	360	757			
8:00 AM	276	293	569	273	285	558	-	-	-	-	-	-	-	-	-	-	-	-	275	289	564			
9:00 AM	179	144	323	189	152	341	-	-	-	-	-	-	-	-	-	-	-	-	184	148	332			
10:00 AM	166	148	314	170	164	334	-	-	-	-	-	-	-	-	-	-	-	-	168	156	324			
11:00 AM	196	170	366	193	194	387	-	-	-	-	-	-	-	-	-	-	-	-	195	182	377			
12:00 PM	204	231	435	240	229	469	-	-	-	-	-	-	-	-	-	-	-	-	222	230	452			
1:00 PM	181	184	365	193	182	375	-	-	-	-	-	-	-	-	-	-	-	-	187	183	370			
2:00 PM	262	203	465	230	244	474	-	-	-	-	-	-	-	-	-	-	-	-	246	224	470			
3:00 PM	378	369	747	419	375	794	-	-	-	-	-	-	-	-	-	-	-	-	399	372	771			
4:00 PM	499	433	932	479	405	884	-	-	-	-	-	-	-	-	-	-	-	-	489	419	908			
5:00 PM	393	431	824	426	426	852	-	-	-	-	-	-	-	-	-	-	-	-	410	429	838			
6:00 PM	276	280	556	281	299	580	-	-	-	-	-	-	-	-	-	-	-	-	279	290	568			
7:00 PM	157	245	402	169	220	389	-	-	-	-	-	-	-	-	-	-	-	-	163	233	396			
8:00 PM	105	132	237	114	169	283	-	-	-	-	-	-	-	-	-	-	-	-	110	151	260			
9:00 PM	85	117	202	86	122	208	-	-	-	-	-	-	-	-	-	-	-	-	86	120	205			
10:00 PM	37	55	92	52	75	127	-	-	-	-	-	-	-	-	-	-	-	-	45	65	110			
11:00 PM	18	35	53	25	37	62	-	-	-	-	-	-	-	-	-	-	-	-	22	36	58			
Total	4,146	4,162	8,308	4,256	4,270	8,526	-	-	-	-	-	-	-	-	-	-	-	-	4,201	4,216	8,417			
Percent	50%	50%		50%	50%		-	-	-	-	-	-	-	-	-	-	-	-	50%	50%		-	-	
AM Peak	07:00	07:00	07:00	07:00	07:00	07:00	-	-	-	-	-	-	-	-	-	-	-	-	07:00	07:00	07:00			
Vol.	411	364	775	382	356	738	-	-	-	-	-	-	-	-	-	-	-	-	397	360	757			
PM Peak	16:00	16:00	16:00	16:00	17:00	16:00	-	-	-	-	-	-	-	-	-	-	-	-	16:00	17:00	16:00			
Vol.	499	433	932	479	426	884	-	-	-	-	-	-	-	-	-	-	-	-	489	429	908			

1. Mid-week average includes data between Tuesday and Thursday.